

URBAN DEVELOPMENT DIRECTORATE (UDD)

Ministry of Housing and Public Works

Government of the People's Republic of Bangladesh

Establishment of Monitoring Well and Field Investigation Report On HYDRO-GEOLOGICAL SURVEYS AND STUDIES UNDER PREPARATION OF DEVELOPMENT PLAN FOR MEHERPUR ZILLA

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Managing Director

Center for Geoservices and Research

2 Acronyms

UDD Urban Development Directirate

CGR Center for Geoservices and Research

VES Vertical Electrical Sounding

EC Electrical Conductivity

TDS Total Dissolved Solid

ASTM American Society for Testing and Materials

3 Introduction

This report describes the establishment of a groundwater monitoring network in Meherpur districts under the hydrogeological surveys and studies in the Preparation of Development Plan for Meherpur Zilla project. Additionally, details of field investigations for water samplings and slug tests have been described. The field investigation on water quality represents for Arsenic concentration the whole project area is free of Arsenic according to WHO and Bangladesh Standard and for salinity of shallow and intermediate aquifer is low to moderate intense. (Source: CGR)

The establishment of these monitoring networks is the first step in a detailed hydrogeological investigation in the study area that will be carried out over the period of more than a year from now. A total of 21 monitoring wells have been drilled and installed at seven (07) locations. At each location, three (03) co-located wells (5 to 10 feet apart) have been installed at different depths. The deepest of each set is about 500 ± 100 feet deep, the intermediate one is about 300 feet deep, and the shallowest one is about 100 feet deep.

Groundwater level as well as water quality in the study area will be monitored for about a year from now in each of these monitoring wells. The data that will be collected will be of outmost importance characterizing the hydrogeological condition in the study area. Detail methodology, locations, and lithological data collected during the drilling of these wells are discussed in the subsequent sections.

4 Methodology:

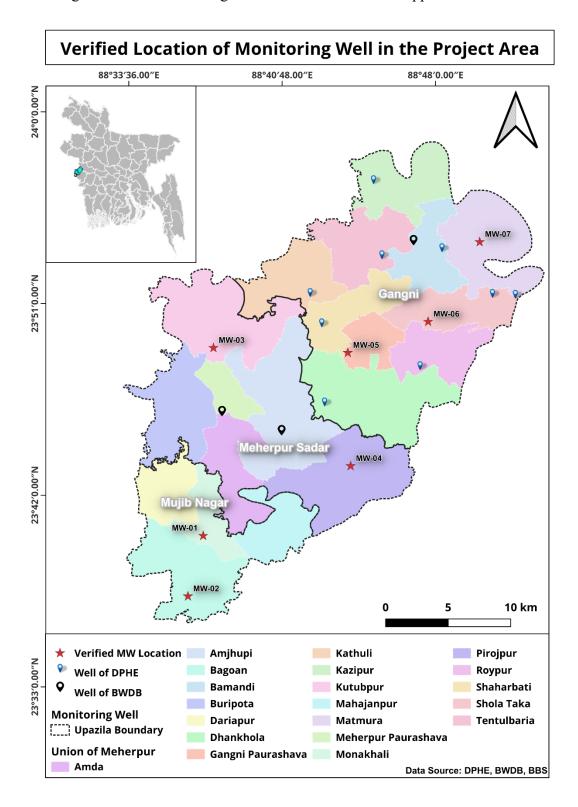
In order to establish the monitoring network in the field a field trip was carried out between 20 February and 22 March 2025. A team consisting of 1 geologist and 1 Civil Engineer worked together in the field for the entire period. The drilling team was employed to drill and install monitoring wells one by one. Detail methodology for site selection, drilling, sampling, logging, and installation of the monitoring network is discussed in the subsequent sections.

4.1 Monitoring Well Installation

4.1.1 Site Selection:

Monitoring well locations were selected first on the basis of Geological, Geomorphological, and hydrogeological variability, and the location of existing data in the study area. Later on, the locations were verified by physical observation and shifted a bit on the basis of local access and

available space for the investigation as well as the permission of the landowners. All the locations are verified finally, and permission is also obtained from the landowners. Locations of the monitoring wells are shown in Figure 1 and in Table 1 in the Appendix.



Map 1 Location map of the monitoring nests (Source: CGR)

4.1.2 Drilling of Monitoring Wells:

Since the groundwater quality in the study area varies with depth, monitoring wells at multiple depth intervals is essential. A total of 21 monitoring wells have been installed at Seven (07) locations (one set of 3 wells, Map 1 & Figure 2). At each location a cluster/nest¹ of three wells (one at around 500±100 feet depth, one at around 300 feet depth and the other at around 100 feet depth, each well will be within 5-10 ft from the other) have been installed as shown in Figure-2.

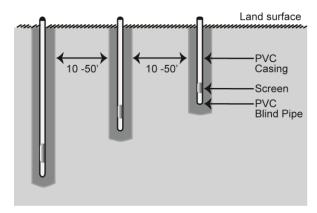


Figure 1 Cross Sectional View of Well Nest/Cluster

Reverse circulation conventional drilling method (Figure 2) was used for drilling the monitoring wells. In this method, drilling fluid enters the hole through the drill pipe and comes up to the surface with a mixture drill cutting through the annulus. Fluid was piped through the pipe using a high-speed mechanical pump. A mixture of water and cow dung was used as drilling fluids. For the deepest piezometer drilling was continued for at least 1000 feet.

4.1.3 Lithological Sampling and Logging:

A well site Geologist was present at each site during drilling, and he was responsible for logging the samples in standard format, collected every 10 feet interval. He logged the lithology in the log sheet provided by the consultancy firm and preserved the samples for further laboratory test i.e. Grain size analysis. The drill cuttings were collected in bucket and preserved in polyethylene bag for further laboratory analysis. The samples were analyzed visually by an onsite geologist and a driller's log was prepared at the field. All the bore logs are presented in appendix 3.

¹ Nest well: A cluster of wells where tubes or pipes are constructed in separate (5-10 feet distance to each other), individual boreholes that are drilled and completed at different depths.



Figure 2 Drilling Procedure of Monitoring well in Mujibnagar Upazila.

4.1.4 Installation of Monitoring Wells

After the drilling was completed, a monitoring well was installed at every drill hole. The deepest monitoring wells have 20 feet screens at the bottom of the well but above 10 to 30 feet blind pipes. The shallower monitoring wells have a 10 feet screen above 10 feet blind pipe. Both the well casing and screen consist of PVC materials (Figure 3). After installing the pipes, gravel packing was done around the well screen. The well annulus was back filled by clays collected during the drilling.



Figure 3 Installation of Monitoring Well in Mujibnagar Upazila

4.1.5 Development of Monitoring Wells

After installation, each monitoring well was developed by both manual pumping for duration of several hours for the shallow wells to tens of hours for the deep well until the EC of the well water was stable and by an electrical compressor. A local hand pump was used for the manual pumping for well development.

4.1.6 Water Level Measurement and Sampling

After the successful development of the monitoring wells groundwater level at the monitoring wells were measured using an electronic groundwater level meter. Afterwards the wells were pumped, and water samples were collected for laboratory analysis. During water sampling a number of onsite geochemical parameters were also measured in the field using field test kits. These parameters include P^{H,} Electrical conductivity (EC), TDS, and Arsenic (Figure 4 & 5). Water levels will be measured automatically in hourly interval in the deep wells using automatic data loggers for a period of one year. In shallow wells water level will be measured bi-weekly using a water level meter for the same period.



Figure 4 Water Sampling and Field Test.



Figure 5 Water Sampling and Field test.

4.2 Water Sampling from Existing Wells

A total of 60 existing wells were sampled in 3 Upazilas. Before sampling each well was purged for 5-10 minutes. Samples were collected in 100 ml plastic bottles. Two samples were collected from each well, one sample was acidified, and the other was non-acidified. Both samples were filtered before filling the sampling bottle. Each sample was given a sampling ID and sample bottle was labeled with ID. In addition to sample collection a number of onsite geochemical parameters were also measured in the field using field test kits. These parameters include P^H,

Electrical conductivity (EC), TDS, and Arsenic (Figure 5). Details of the sample locations and field parameters are given in Table 2 in Appendix.

a) 5

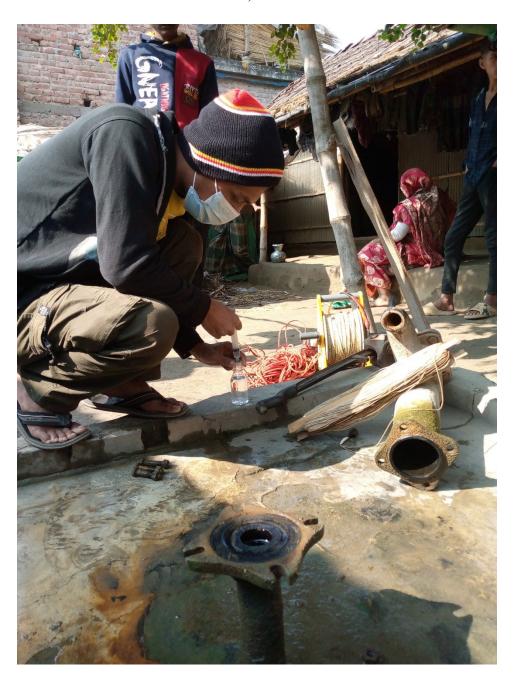


Figure 6 Water Sampling from Existing Well

4.3 Slug Test

Since pump tests are very expensive, they are usually carried out at only a few locations, providing very sparse data on the aquifer properties. A cheap alternative of pump test is slug test. For high density coverage of hydraulic conductivity data slug test will be performed in a large number of wells throughout the study area. Slug test is a field method where a slug (usually a

rod) is inserted in a well below the water table, which causes an instantaneous rise of water level in the well. Dissipation of the water level in the well is then recorded, usually by an automatic water level logger (Figure 7). The temporal rate of this water level declination provides information on the hydraulic conductivity and specific yield/storage of the aquifer surrounding the well. This is a quick but accurate method of estimating hydraulic conductivity in any small diameter tube wells.



Figure 7 Automatic data logger

A slug test is a controlled field experiment, performed by groundwater hydrologists to estimate the hydraulic properties of aquifers and aquitards, in which the water level in a control well is caused to change suddenly (rise or fall) and the subsequent water-level response (displacement or change from static) is measured through time in the control well and one or more surrounding observation wells (Figure 8 &9).



Figure 8 Slug test in the monitoring well

Slug tests are frequently designated as rising-head or falling-head tests to describe water-level recovery in the control well following test initiation. Other terms sometimes used instead of slug test include baildown test, slug-in test and slug-out test. The goal of a slug test, as in any aquifer test, is to estimate hydraulic properties of an aquifer system such as hydraulic conductivity.



Figure 9 Slug Test in Existing Well

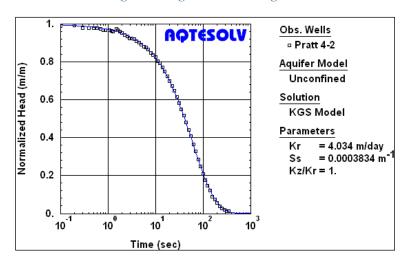
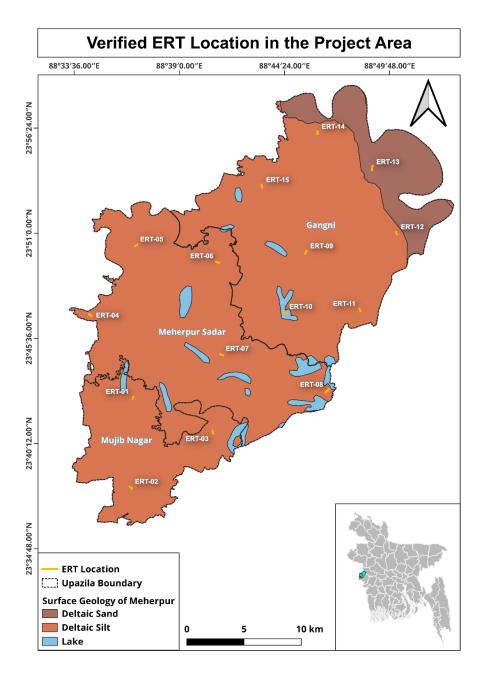


Figure 10 Estimation of aquifer properties from time-displacement data collected during an overdamped slug.

In addition to the 21 monitoring wells that have been installed under this study, slug test have been carried out in at least 60 locations in the existing hand tube wells throughout the study area.

4.4 Electrical Resistivity Tomography

Boreholes provide direct information about the subsurface. However, drilling boreholes is expensive and their density in an area is usually low resulting in a spare point data about the subsurface geology. Interpolation of these sparse data for mapping subsurface geology/aquifers can be erroneous since usually there are data gaps over a large area between each borehole. Geophysical methods can be very useful in minimizing the data gap. In this study, Electrical Resistivity Tomography (ERT) will be conducted in a total of 15 locations in three Upazilas, Map 3.



Map 2 Location Map of ERT (Source: CGR)

4.4.1 METHODOLOGY

Geo-electrical resistivity surveys have long been used for geo-engineering studies, and the method is found to be very successful. Geophysical methods are now widely used in solving complicated geological, hydrological, and environmental problems. Among all the geophysical methods, electrical and electromagnetic techniques are the most popular in groundwater exploration, geotechnical investigations, disaster science due to the close inter-relationship among electrical conductivity, hydrogeological properties of the aquifer such as porosity, clay content, mineralization of the groundwater, degree of water saturation, and lithology. Several case histories conducted in different parts of the world proved that the conventional direct current (DC) resistivity method is one of the most effective tools to decipher the underlying soil conditions in complicated geological setting.

4.4.2 RESISTIVITY OF ROCKS

The resistivity of rocks varies considerably with lithology. The resistivity of rocks varies considerably with lithology. Sediments comprising aquifers are sands of various grain sizes. The electrical conductivity of these sediments depends on the salt concentration of the pore space water they contain. Clay and silt, rich in water-soluble minerals, have low resistivity even when their water content is low. The conductivity of sand and gravel is exclusively the consequence of their pore space water content, as they are composed of electrically non-conducting minerals. Consequently, sand and gravel show very high specific resistivity above groundwater level, and lower values below it. Resistivity is usually the most important property in determining the flow of electric current.

4.4.3 RESISTIVITY PRINCIPLES

In the resistivity method, artificially generated electric currents are introduced into the ground, and the resulting potential differences are measured at the surface. Generally, actual resistivity values are determined from apparent resistivity, which are computed from the measurements of current and potential differences between two pairs of electrodes placed on the ground surface. Two main types of procedures related to the resistivity survey are: vertical electrical sounding (VES) and constant separation traversing (CST). In groundwater exploration, vertical electrical sounding is widely used to identify the aquifer position, their lateral extent, variations in thickness, and water quality.

The resistivity of a material is defined as the resistance in ohms between the opposite faces of a unit cube of the material. For a conducting cylinder of resistance ΔR with a cross-sectional area ΔA and a length ΔL (Fig. 1), the resistivity of the cylinder can be expressed as:

$$\rho = \Delta R. \Delta A / \Delta L$$

The SI unit of resistivity is ohm-meter (Ω -m). The reciprocal of resistivity is termed as the conductivity and the SI unit of conductivity is ohm per meter or Siemens. The Ohm's law, which states that temperature remaining constant, the potential difference 'V' across a current bearing conductor is given by the product of the current 'I' and the resistance 'R' of the conductor.

$$V=IR$$
 (1)

Let the conductor be a plate of thickness 'L' and area of cross-section 'A', then

$$R = \rho L/A \tag{2}$$

Where ρ is the resistivity of the plate

Putting equation (2) in equation (1) we get,

$$V = I\rho L/A,$$
 Or, $V = j\rho L$ [j =I/A= current density] (3)

If 'L' is very small, the potential difference 'V' will also be small Δv , then the ratio $-\Delta v/\Delta l$ is given by the potential gradient E

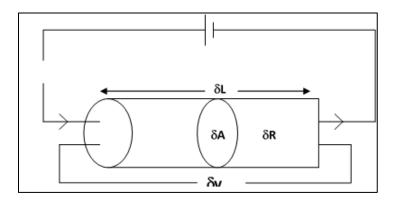


Figure 11 The parameters used in defining resistivity

Therefore,
$$-\Delta v/\Delta l = E = j\rho$$
 or

$$j = E/\rho = \delta E \tag{4}$$

Where, 6 is the conductivity of the plate.

4.4.4 RESISTIVITY OF HOMOGENEOUS ISOTROPIC MEDIUM

The simplest approach to the theoretical study of earth resistivity measurements is to consider first the case of a completely homogeneous isotropic path. An equation giving the potential about a single point source of current on the spherical surface can be developed from ohm's law. In homogeneous isotropic ground where there is a point source of current below the ground surface, the current radiates equally in all directions. Hence the equipotential surfaces are spherical with center at source point. For two such equipotential surfaces very near to each other the potential difference would be v_1 - v_2 = Δv between them.

If the radial distance between them is Δr , then potential gradient E at any point between them is

$$-\Delta v/\Delta r = E$$

The current density 'j' at any point on the equipotential surfaces would be

$$i = I/4\pi r^2$$

By ohms law,

$$J = I/4\pi r^{2} = 6E = -6 \Delta v/\Delta r$$

$$Or, \Delta v/\Delta r = -1/6. I/4\pi r^{2}$$

$$\therefore \Delta v = -1/6. I/4\pi r^{2}.\Delta r$$
(5)

Now, integrating the equation (5) we get,

$$V = 1/6. \text{ I}/4\pi r + C$$

$$V = \rho \text{I}/4\pi r \qquad (6)$$
When $r = \infty \rightarrow C = 0$.

If the point source of current is at the ground surface, then the current will flow hemispherically, then equation (6) can be expressed as

$$V = \rho I / 2\pi r \tag{7}$$

Potential functions are scalars and so, may be added arithmetically. If there are several sources of current rather than the single source assumed so far, the total potential at an observation point may be calculated by adding the potential contributions from each source independently. Thus,

for n current sources distributed in a uniform medium, the potentials at an observation point, M will be-

$$V_{M} = \rho/2\pi \left[I_{1}/a_{1} + I_{2}/a_{2} + \dots + I_{n}/a_{n} \right]$$
 (8)

Where, I_n is the current from the n^{th} in a series of current electrodes and a_n is the distance from the n^{th} source to the point at which the potential is being observed.

Equation (8) is of practical importance in the determination of earth resistivities. The physical quantities measured in a field determination of resistivity are the current, I, flowing between two current electrodes; the difference in potential ΔV , between two measuring points and the distance between the various electrodes.

When there are two current electrodes (A&B) on ground surface and the distance between two current electrodes is finite (Figure 12), the potential at any nearby surface point will be affected by both current electrodes.

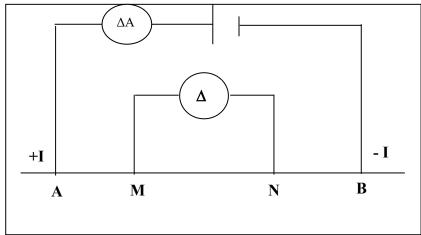


Figure 12 Generalized form of the electrode configuration used in resistivity measurements

The potential V_m at an internal potential electrode M_c is the sum of the potential contributions V_A and V_B from the current source at A and the sink at B.

$$V_M = V_A - V_B$$

From the equation (7) we get,

$$V_{M} = \rho I/2\pi (1/AM - 1/BM)$$
 (9)

Similarly,

$$V_{N} = \rho I/2\pi (1/AN - 1/BN)$$
 (10)

Absolute potentials are difficult to monitor so the potential difference ΔV between two electrodes 'M' and 'N' is measured:

$$\Delta V = V_M - V_N = \rho I/2\pi \left\{ (1/AM - 1/BM) - (1/AN - 1/BN) \right\}$$
Thus $\rho = 2\pi \Delta V/I \left\{ (1/AM - 1/BM) - (1/AN - 1/BN) \right\}$ (11)

The equation (11) is applied for the ordinary four terminal arrays in measuring the earth resistivity in the field. Where the ground is uniform, the resistivity calculated from equation (11) should be constant and independent of both electrode spacing and surface location. When subsurface in homogeneities exist, the resistivity will vary with the relative positions of the electrodes. Any computed value is then known as the apparent resistivity (ρ_a) and will be a function of the form of the in homogeneity. Equation (11) is basic equation for calculating the apparent resistivity for any electrode configuration.

The arrangement of current and potential electrodes on or in the ground for the purpose of making an electrical survey is called electrode configuration. The current electrodes are generally placed on the outside of the potential electrodes.

Based on the position of current or potential electrodes and variation in distance between them, a variety of electrode configurations are possible of which some are mentioned below:

- I. Wenner configuration
- II. Schlumberger configuration
- III. Dipole-dipole configuration

The choice of array and distance between the electrodes is very important for obtaining the best possible information of the subsurface geology of a given area. For ERT, Wenner configuration is preferred.

The Wenner configuration, in which potential difference is measured, is one of the simpler and most commonly used electrode arrays for determining resistivity. This is a symmetrical configuration consisting of four electrodes, the outer two electrodes are current electrodes and the inner two electrodes are potential electrodes. The distance between any two adjacent electrodes is called the array spacing and is equal and usually denoted by 'a' along a straight line (Figure 13).

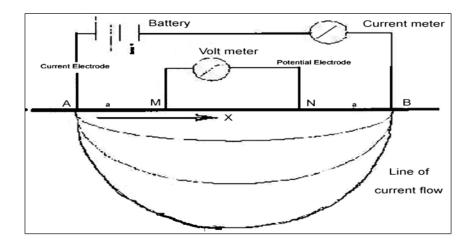


Figure 13 Wenner electrode configuration showing equal spacing

In spite of the simple geometry, this arrangement is quite often inconvenient for fieldwork. This arrangement has some disadvantages from a theoretical point of view. For depth exploration using the Wenner configuration, the electrodes are expanded about a fixed center, increasing the spacing 'a' insteps. In case of lateral exploration of mapping, the spacing remains constant and all four electrodes are moved along the line, then along another line and so on.

The apparent resistivity with the Wenner array is given by-

$$\rho a = 2\pi a \cdot \Delta V/I$$

Where,

 ΔV = Potential difference in volts between V1 and V2;

I = Current in ampere;

a = Array spacing;

 $\rho a = Apparent resistivity;$

The geometric factor of Wenner configuration is $2\pi a$;

the resistivity of subsurface rock depends on this factor.

4.4.5 BASIC IDEA AND FUNDAMENTALS OF RESISTIVITY SURVEY

Geo-electrical resistivity survey has long been used for ground water survey and the method is found to be very successful. As a preliminary step for the development of ground water, geo electrical resistivity survey proved to be very effective- (Bugg & Lloyed, 1976; Serres, 1969; Urish & Frohlich, 1990; Woobaidullah et al, 1996). In resistivity method, artificially generated electric currents are introduced into the ground and the resulting potential differences are measured at the surface. Generally actual resistivities are determined from apparent resistivities, which are computed from the measurements of current and potential differences between two pairs of electrodes placed in the ground surface. The procedure involves measuring a potential difference between two potential electrodes (M & N) resulting from an applied current through two other current electrodes (A & B) outside but in line with the potential electrodes. Thus, the measured current and potential differences yield an apparent resistivity over an unspecified depth. If the spacing between the electrodes is increased penetration of the electric field also increases and a different apparent resistivity is obtained. Two main type resistivity surveys are: Vertical Electrical Sounding and Constant Separation Traversing/ Profiling (CST).

Vertical Electrical Sounding sees the current and potential electrodes at the same relative spacing and the whole spread is progressively expanded about a fixed central point. Consequently, readings are taken as the current reaches progressively greater depths. In this case Schlumberger configuration is favored. Constant Separation Traversing is used to determine the lateral variations of resistivity. The current and potential electrodes are maintained at a fixed separation and progressively moved along a profile. In this case Wenner configuration is favored.

Electric Resistivity Tomography (ERT) is a robust and well-consolidated method for near-surface geophysics, with a wide spectrum of applications in the geological, engineering and environmental sciences. Technological advances (e.g., multi-channel arrays, innovative sensors) and novel tomographic algorithms for data inversion have rapidly transformed ERT into one of the most employed geophysical methods. In essence, the survey procedures are similar to CST, but with each increment in electrode spacing, the survey direction is reversed.

As for data processing, an inversion model is conducted. The least-squares method is used to calculate certain physical characteristics of the subsurface, the "model parameters", from the apparent resistivity measurements. The "model parameters" are set by the way we slice and dice the subsurface into different regions. The method most commonly used in 2-D interpretation is a purely cell-based model where the subsurface is subdivided into rectangular cells. The positions

of the cells are fixed and only the resistivity values of cells are allowed to vary during the inversion process. The model parameter is the resistivity of each cell.

4.4.6 ERT DATA ACQUISITION

To define the subsurface resistivity condition Wenner configuration (WN) has been used. Four equally spaced electrodes are used in WN where two outer electrodes (A & B) acted as current electrodes (source) and two inner electrodes (M & N) acted as potential electrodes (Figure 14). The array spacing expands about the array midpoint while maintaining an equivalent spacing between each electrode (Keller, 1996). The resistivity measured by this configuration can be determined from the readings of current intensity (I) and the potential difference between M and N (Δ V) values as follows:

$$\rho A = k \frac{\Delta V}{I}$$

Where ρA is the apparent resistivity, k is the geometric factor as defined by the following expression:

$$k = \frac{2\pi}{(\frac{1}{AM} - \frac{1}{BN} - \frac{1}{AN} + \frac{1}{BN})}$$

The Wenner array employed in this study is one of the most commonly used arrangements, in the field.

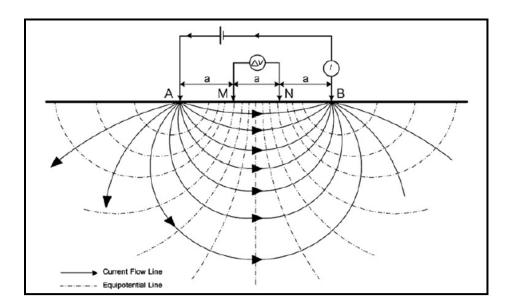


Figure 14 Wenner array ("a" is electrode spacing) and distribution of electric field underneath. (After Todd and Mays, 1980; Source: Wiwattanachang & Giao, 2011)

A spacing "a" separates the four electrodes equally. Wenner array's geometric coefficient is equal to 2a (Wiwattanachang & Giao, 2011). As a result, the apparent resistivity is determined as follows:

$$\rho A = 2\pi a \frac{\Delta V}{I}$$

Wenner configuration with a spread length of 90 m maximum is planned to execute where the electrode spacing was 3, 6, 9, 12 and 15 m respectively in all 20 sites in the investigated area.

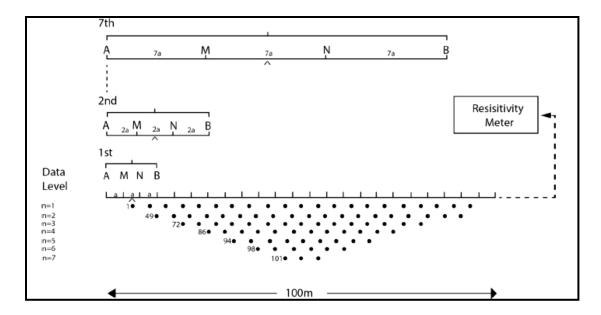


Figure 15 Measurement sequence for constructing a pseudo section.

4.4.7 ERT DATA PROCESSING

The measured data were converted into a suitable (.dat) format and therefore inverted using the smoothness-constrained least-squares inversion method in the Res2Dinv software (Loke and Barker, 1996; Sasaki 1992; deGroot-Hedlin and Constable 1990). The smoothness-constrained least-squares method is based on the following equation:

$$(\mathbf{J}^{\mathsf{T}}\mathbf{J} + u\mathbf{F}) \mathbf{d} = \mathbf{J}^{\mathsf{T}}\mathbf{g}$$

Where, F = fxfxT + fzfzT; fx = horizontal flatness filter; fz = vertical flatness filter.

J = matrix of partial derivatives; u = damping factor; d = model perturbation vector; g = discrepancy vector.

Total 15 nos. of ERT will be conducted in the project area in the month of June 2025.

5 Discussion

A total of 21 monitoring wells have been drilled and installed at three locations. At each location, three (03) co-located wells (5 to 10 feet apart) have been installed at different depths. The deepest of each set is about 500±100 feet deep, the intermediate one is about 300 feet deep and the shallowest one is about 100 feet deep.

The field investigation on water quality represents for Arsenic concentration the whole project area is free of Arsenic for salinity of shallow to intermediate aquifer low to moderate intense throughout the project area.

The other investigations like grain size analyses have been completed. Major Anion, Cation, Trace element analyses are going on in laboratory and it will take more than a month and will be included in the next phase of reporting. The VES data collection will be completed by 25th June 2025.

6 APPENDICES

APPENDIX: Table 1 Location of Monitoring Wells.

MW No	MW No Lat	Lng	Location	Union	Upazila	Zila
MW-01	MW-01 23.668205 88.618213 Vobanipur	88.618213	Vobanipur Primary School, Monakhali Union, Mujib Nagar, Meherpur	Monakhali	Mujib Nagar	Meherpur
MW-02	23.620828	88.606308	WW-02 23.620828 88.606308 Anandabas Dakkhin Para Govt Primary School, Bagoan Union, Mujib Nagar, Meherpur Bagoan	Bagoan	Mujib Nagar	Meherpur
MW-03	WW-03 23.815262 88.626262 Ujalpur Hi,	88.626262	Ujalpur High School, Kutubpur Union, Meherpur Sadar, Meherpur	Kutubpur	Meherpur Sadar Meherpur	Meherpur
MW-04	MW-04 23.722818 88.733614 Mominpur	88.733614	Govt Primary School, Pirojpur Union, Meherpur Sadar, Meherpur	Pirojpur	Meherpur Sadar Meherpur	Meherpur
MW-05	23.811422	88.731416	MW-05 23.811422 88.731416 Bashbaria Govt Primary School, Gangni Paurashava, Gangni, Meherpur	Gangni Paurashava Gangni		Meherpur
90-WW	23.835775	88.794261	MW-06 23.835775 88.794261 Baniapukur Govt Primary School, Shola Taka Union, Gangni, Meherpur	Shola Taka	Gangni	Meherpur
MW-07	MW-07 23.898125 88.8346 Motmura (88.8346	Motmura Govt Primary School, Matmura Union, Gangni, Meherpur	Matmura	Gangni	Meherpur

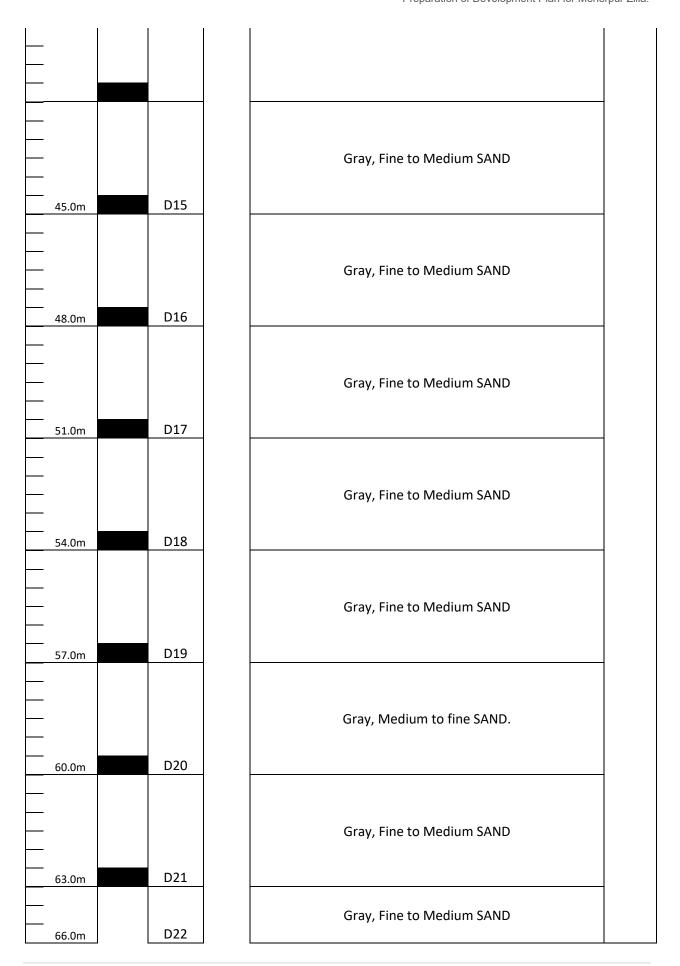
APPENDIX: Table 2 Field Parameter of Water Sample

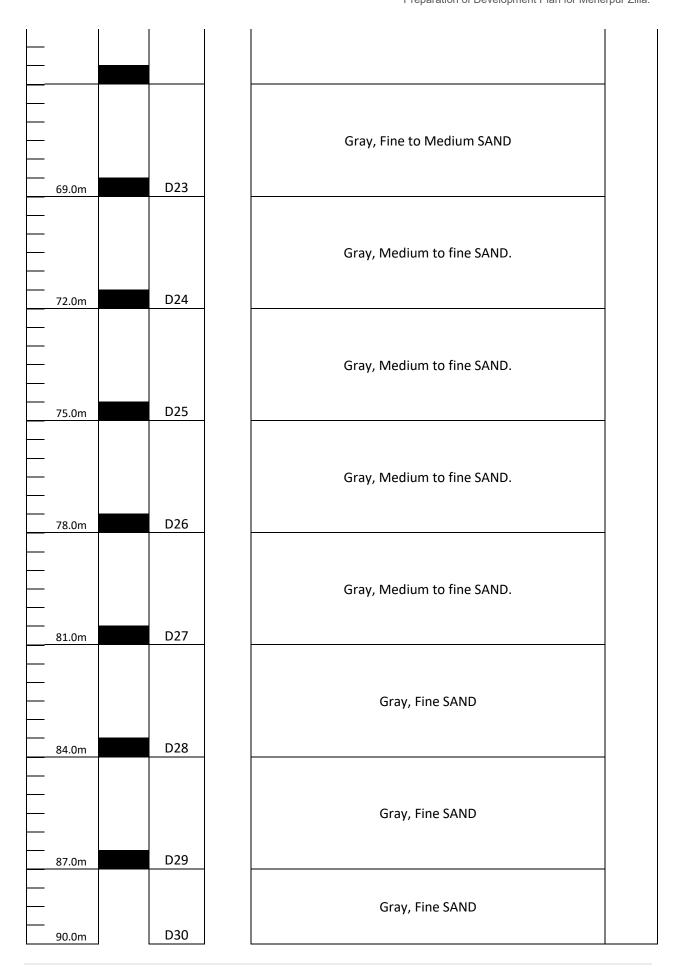
Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	Done	2 5 0												
Done	Done		Done	Done		Done	Done	Done	- O	Done	Done		- 0	Done		Done	Done	200	Done	Done	Done		Done	Done		Done	Done		Done	Done		Done		- Done		Done		Done											
0.1	0.1	0.25	0.09	0.13	0.07	0.2	0.1	0.12	0.12	0.18	0.08	0.1	0.05	0.04	0.25	0.08	0.08	0.06	0.09	0.15	0.05	0.22	0.4	0.08	0.1	0.07	0.04	0.26	0.3	0.2	0.12	0.1	0.08	0.24	0.22	0.06	0.1	0.27	0.22	0.17	0.4	0.25	0.08	0.27	0.24	0.1	0.28	0.05	2.5
5.5	5.05	6.52	6.4	6.1	5.55	5.7	6.8	6.46	5.76	5.78	5.8	5.97	5.6	6.5	4	6.75	6.64	5.9	5.91	6.33	4.7	5.9	4.34	6.52	4.82	5.8	5.03	4.8	4.91	4.75	3.99	4.91	3.95	4.63	4.93	3.63	5.1	3.66	4.31	2 2 2	5.46	5.83	5.46	5.45	4.3	5.07	5.41	5.77	00:0
160	100	90	09	09	50	160	160	120	70	45	100	100	70	100	45	20	09	160	60	09	50	120	09	20	09	45	20	130	06	100	202	80	130	65	75	90	65	20	100	100	120	100	80	230	70	180	150	50	200
26.7	21.6		26.6		25.3		26.3		28.5		25.1		21.1	21.7		19.2	27.6	21.3	243	24.1	18.6		900	23.1		26.7	26.6		23.4	25.3	24		24.9	26.6		27.2	24.1		21.9	23.9		27.2		25.1		26.2	. 00	7.0.7	
10	10		20		0		10		0	-	0		0	25		0	0	10	, c	0	10			0		0	0 0	,	10	10	0		9 ,	52	. 5	2 0	0		25	01		22		- 52		10		0 1	
096	970		980		244		237		238		255		273	970		276	980	261	- 080	970	1010		- 60	252		244	243	060	940	006	006		880	880		910	970		096	910		006		- 080		920	. 00	066	
1175	671		802		818		982		1277		1507		1004	877		1425	2000	808	750	1326	1255		4500	1430		775	1032	100	854	1108	200		648	928		1007	941		684	745		869		- 642	! .	817	. 0	069	
6.82	7.14		7.07		6.98		6.87	-	6.97	-	6.88		6.9	7.14		7.76	7.15	6.98	7.25	7.04	6.76		- 2	6.65		6.87	6.87	20:7	7.31	96.9	7.1		7.25	7.06	. 00	6.87	6.92		7.21	7.07		96.9		7.08		7.07		7.7	
88.592133	88.61218	88.6228	88.63687	88.612162	88.623	88.60778	88.60473	88.612313	88.67006	88.68498	88.68331	88.66432	88.651614	88.63288	88.643177	88.6308	88.61638	88.59048	88.50715	88.63041	88.66794	88.64353	88.686492	88.67822	88.71146	88.71795	88.771158	88.812794	88.71352	88.819782	88.792043	88.76188	88.74669	88.71097	88.7304	88 717427	88.66147	88.70391	88.72276	88.74047	88.74332	88.77371	88.80604	88.80818	88.81242	88.7959	88.825534	88.85651	1000000
23.652351	23.62967	23.64574	23.65352	23.667146	23.71415	23.71214	23.69647	23.679816	23.65417	23.67485	23.69738	23.68206	23.707255	23.73329	23.762553	23.77949	23.79721	23.74122	23.75507	23.81229	23.81957	23.86417	23.828382	23.74912	23.7358	23.706131	23.711331	23.774933	23.78925	23.798092	23.824077	23.81958	23.82054	23.83114	23.8563	23.859341	23.86018	23.898735	23.90084	23.90475	23.95938	23.945	23.94422	23.82876	23.87088	23.8526	23.845624	23.83431	60:000010
Bagoan	Bagoan	Bagoan	Monakhali	Monakhali	Monakhali	Dariyapur	Dariyapur	Dariyapur	Mohajonpur	Mohajonpur	Mohajonpur	Amda	Amda	Amda	Pourosova	Pourosova	Pourosova	Buripota	Burinota	Kutubbur	Kutubpur	Kutubpur	Amjhupi	Amihupi	Baradi	Pirojpur	Baradi	Dhankhola	Dhankhola	Raypur	Raybur	Pourosova	Pourosova	Saharbati	Saharbati	Kathuli	Kathuli	Kathuli	Tetulbaria	Tetulbaria	Kazipur	Kazipur	Kazipur	Bamandi	Bamandi	Sholotaka	Sholotaka	Sholotaka	Matingla
Mujibnagar	Meherpur Sadar	Mehernir Sadar	Meherpur Sadar	Meherpur Sadar	Meherpur Sadar	Meherpur Sadar	Meherour Sadar	Meherpur Sadar	Meherpur Sadar	Meherpur Sadar	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Gangni	Cargin																									
Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Mehernin	Meherbur	Meherpur	Meherpur	Meherpur	Meherbur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Mehernir	Meherpur	Meherpur	Mehernur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Meherpur	Mehernir	Meherpur	Meherpur	Meherpur	Mehernur	Motorphy												
1	2		3		4		2		9		7		8	6		10	11	12	- 13	2 4	15		- 97	17		18	90 00	- 07	21	22	23		24	25	. 8	8 %	28		53	30		31		- 68		33	. 3	5 ,	
-	2	3	4	2	9	7	8	6	10	11	12	13	14	15	16	17	18	19	21 50	22	23	24	25	27	28	29	30	32	33	34	36	37	38	40	41	42	44	45	46	47	49	20	51	25	24	22	29	28	200

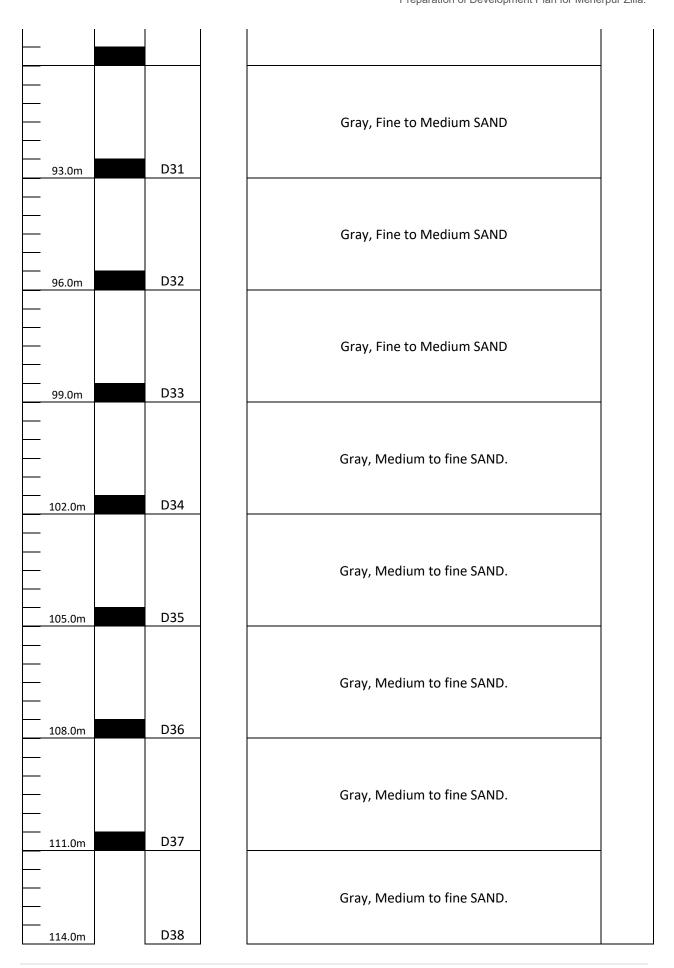
APPENDIX: Table 3 Bore holes logs

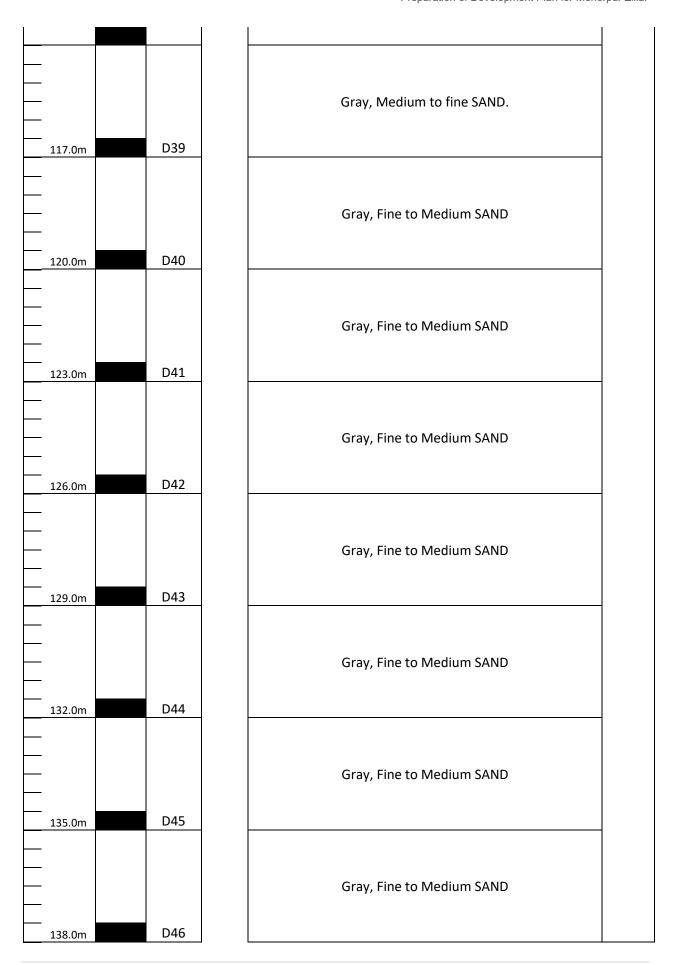
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-01D Location: Bhabanipur Govt. Primary School, Mujibnagar Co-ordinate:23.668205, 88.618214 GEOSERVICES & RESEARCH Depth of Boring:150 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 20/02/2025 Depth Bellow GL (m) Layer Change Sample No Thickness (m) Lithologic Description Brownish Gray, Silty CLAY 3.0m D1 Brownish Gray, Silty CLAY D2 6.0m Brownish Gray, Silty CLAY D3 9.0m Gray, Very Fine to Fine SAND. D4 12.0m Gray, Very Fine to Fine SAND. D5 15.0m D6 Gray, Very Fine to Fine SAND. 18.0m

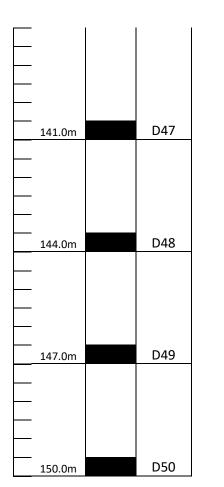
<u></u>			
<u></u>			
<u> </u>			
			Gray, Very Fine to Fine SAND.
			, ,
		D7	
21.0m		υ/	
			Gray, Very Fine to Fine SAND.
			
24.0m		D8	
			Gray, Fine SAND.
			5.5,,5.
		D0	
27.0m		D9	
			Gray, Fine SAND.
30.0m		D10	
<u></u>			
			Gray, Fine SAND.
			,
		D11	
33.0m		D11	
			Gray, Fine SAND.
36.0m		D12	
			Grav. Fine to Medium SAND
			2.27,
		D13	
		D13	
39.0m			
36.0m		D12	Gray, Fine SAND. Gray, Fine to Medium SAND

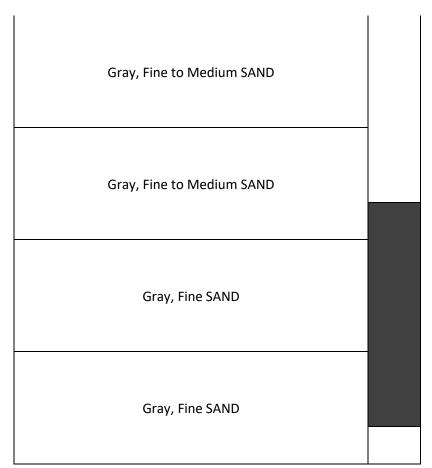




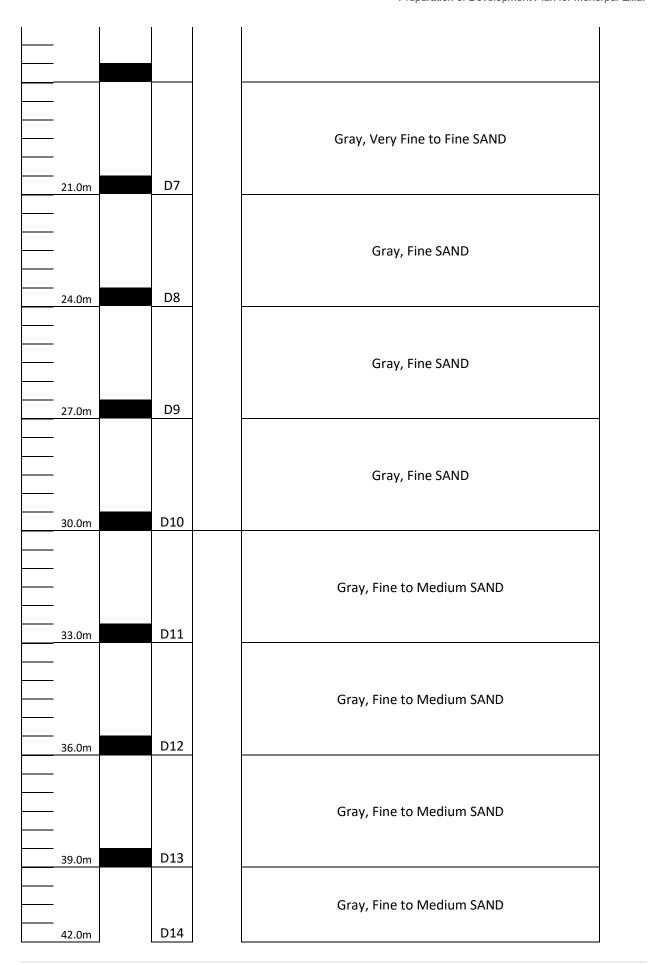


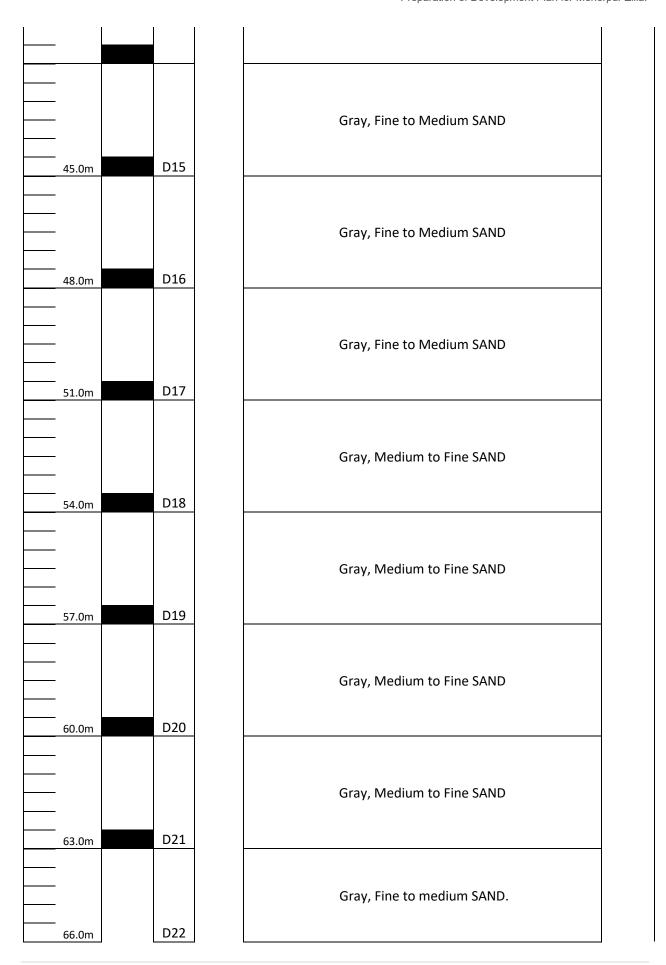


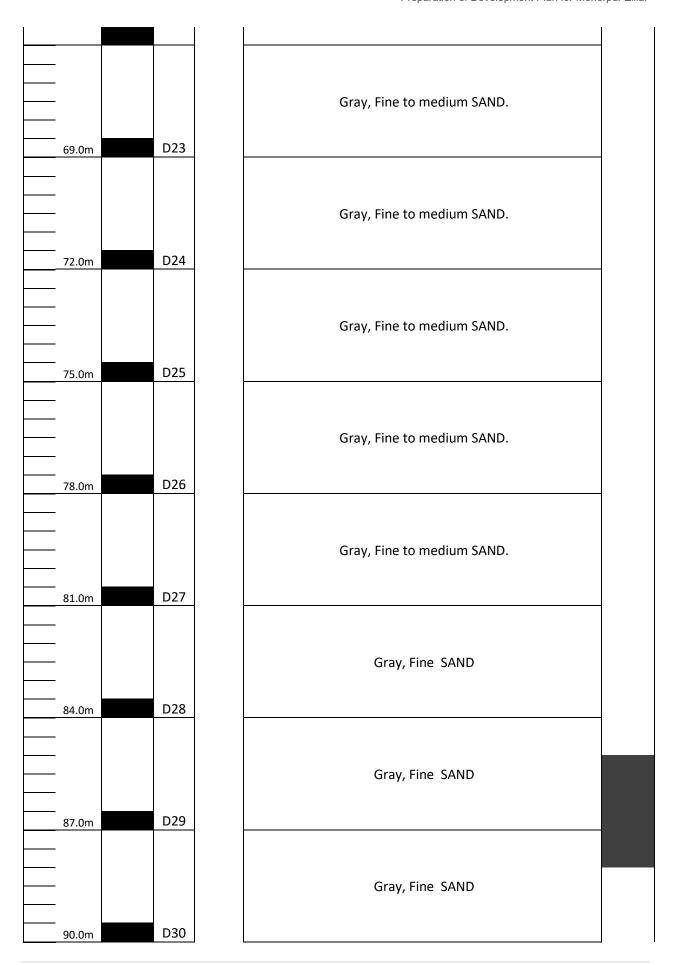




Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-011 Location: Bhabanipur Govt. Primary School, Mujibnagar Co-ordinate:23.668205, 88.618214 GEOSERVICES & RESEARCH Depth of Boring: 90 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 23/02/2025 Type of Sample Layer Change Sample No Depth Bellow Thickness (m GL (m) Lithologic Description Brownish Gray, Silty CLAY D1 3.0m Brownish Gray, Silty CLAY D2 6.0m Brownish Gray, Silty CLAY D3 9.0m Gray, Very Fine to Fine SAND D4 12.0m Gray, Very Fine to Fine SAND D5 15.0m Gray, Very Fine to Fine SAND D6 18.0m







Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-01S Location: Bhabanipur Govt. Primary School, Mujibnagar Co-ordinate:23.668205, 88.618214 IGEOSERVICES & RESEARCH Depth of Boring: 30 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 25/02/2025 Type of Sample Depth Bellow GL (m) Layer Change Sample No Thickness (m Lithologic Description Brownish Gray, Silty CLAY 3.0m D1 Brownish Gray, Silty CLAY D2 6.0m Brownish Gray, Silty CLAY D3 9.0m Gray, Very Fine to Fine SAND. D4 12.0m Gray, Very Fine to Fine SAND. D5 15.0m Gray, Very Fine to Fine SAND. D6 18.0m

	D7	Gray, Very Fine to Fine SAND.	
	D8	Gray, Fine SAND	
27.0m	D9	Gray, Fine SAND	
30.0m	D10	Gray, Fine SAND	

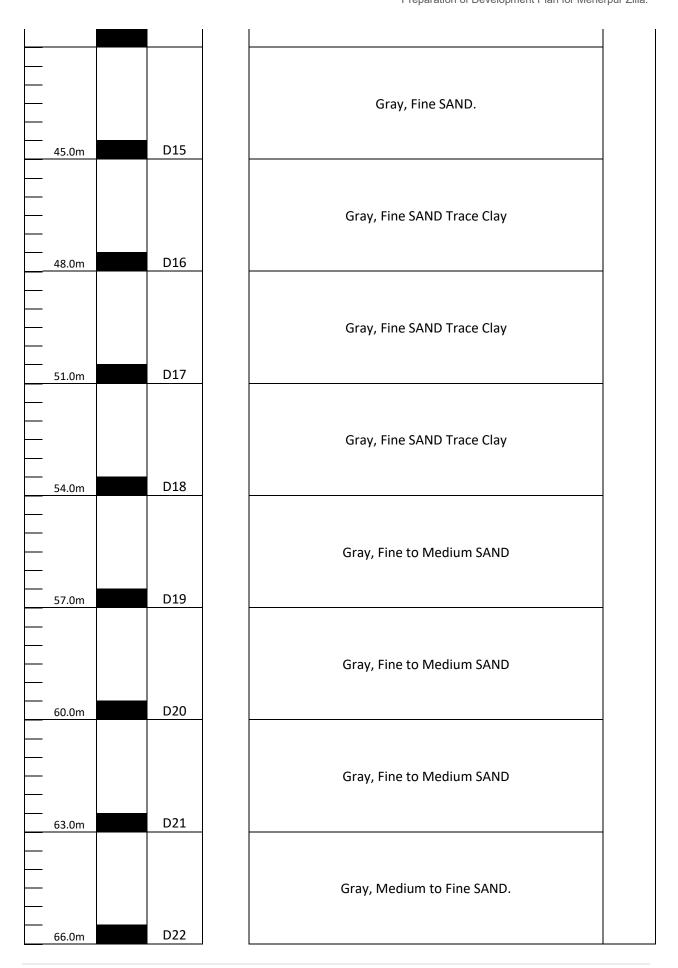
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla

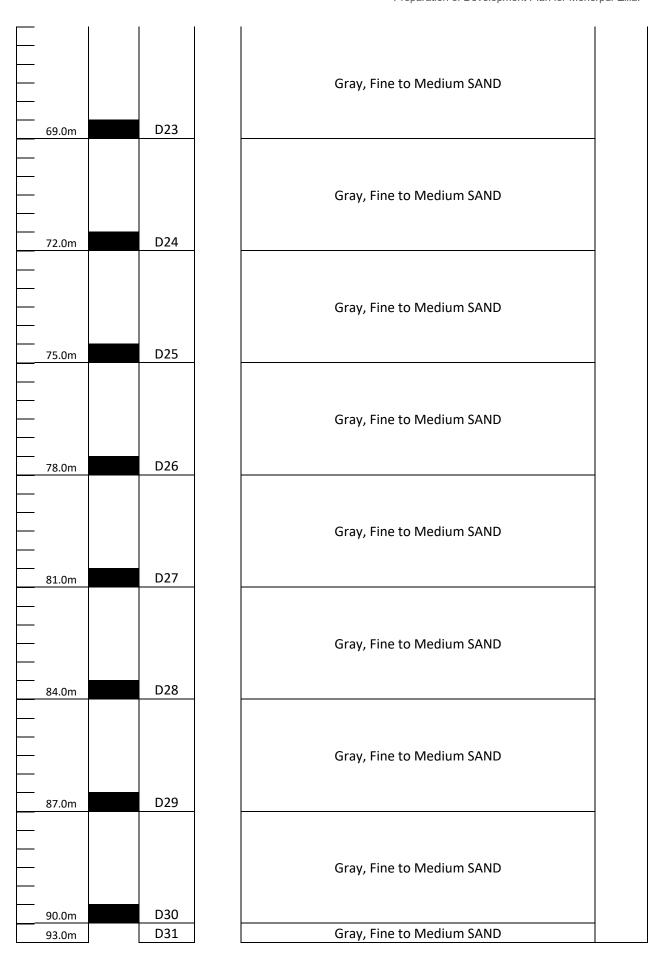
	*	udies under Development Plan for Meherpur Zilla	
Client: Urban Developme	nt Directorate	(UDD)	
Bore Hole ID: MW-02D Location: Anondobas Dok Co-ordinate: 23.620828, 8 Depth of Boring:132 Meto	88.606309	Primary School, Mehering GEOSERVICES & RESE	VSCH
Ground Water Level: Me			
Method of Boring: Rotary	Wash Boring		
Boring Diameter: 1.5"			
Date: 25/02/2025	01 01		a)
Depth Bellow GL (m) Type of Sample	Sample No Thickness (m)	Lithologic Description	Layer Change
_			
		Brownish Gray, Silty CLAY	
3.0m D1			
		Brownish Gray, Silty CLAY	
6.0m D2		Brownish Gray, Very Fine SAND	
9.0m D3		Gray, Very Fine to Fine SAND.	
12.0m D4		Gray, very time to time SAND.	
		Gray, Very Fine to Fine SAND.	
15.0m D5			
		Gray, Very Fine to Fine SAND.	

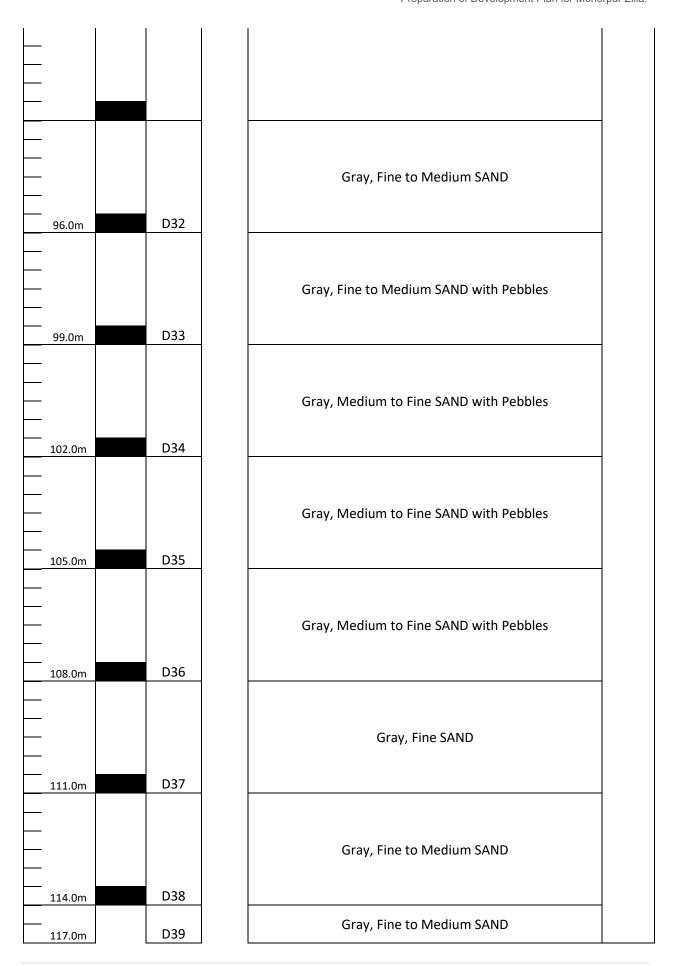
18.0m

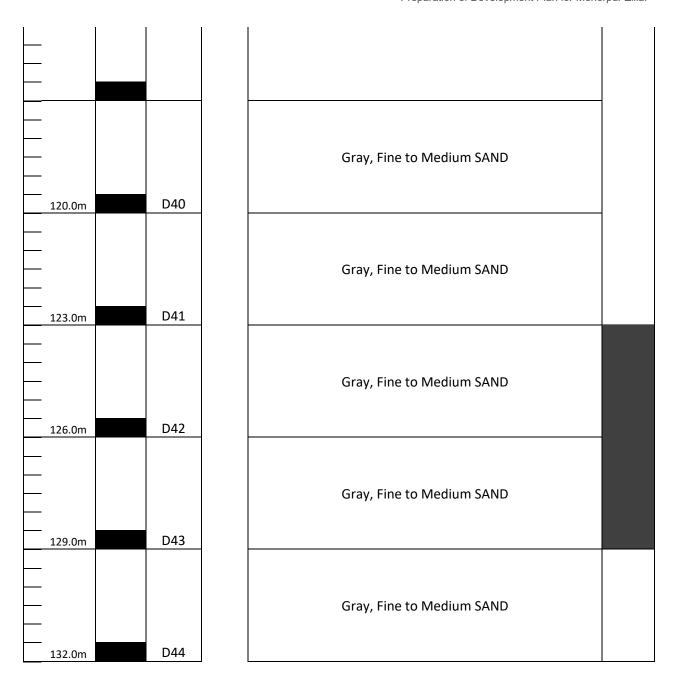
D6

	D7	Gray, Very Fine to Fine SAND.
	D8	Gray, Fine SAND.
	D9	Gray, Fine SAND.
	D10	Gray, Fine SAND.
	D11	Gray, Fine SAND.
	D12	Gray, Fine SAND.
	D13	Gray, Fine SAND.
42.0m	D14	Gray, Fine SAND.









I GEOSERVICES & RESEARCH

Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD)



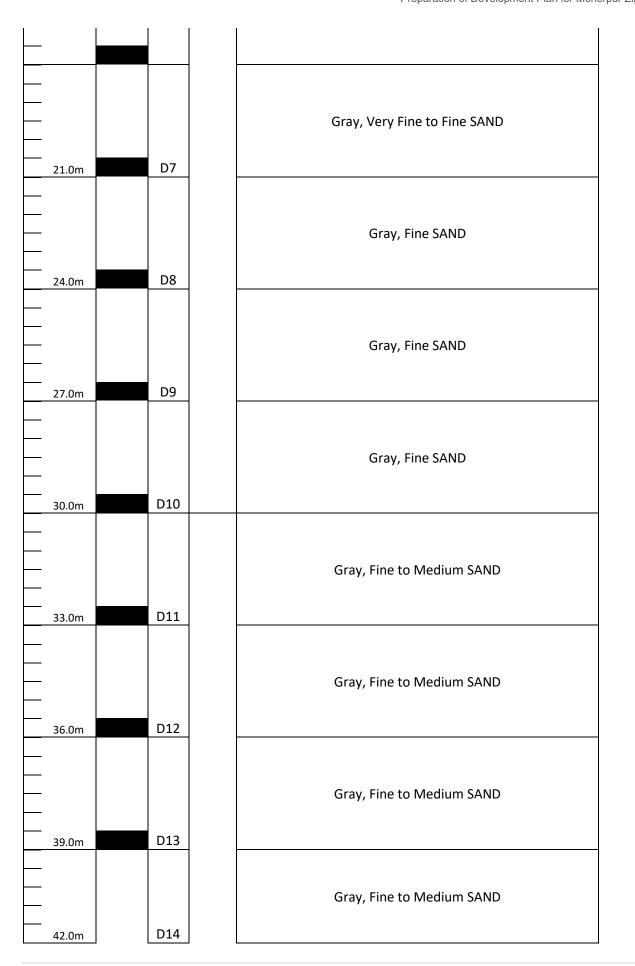
Location: Anondobas Dokhinpara Govt. Primary School, Meh

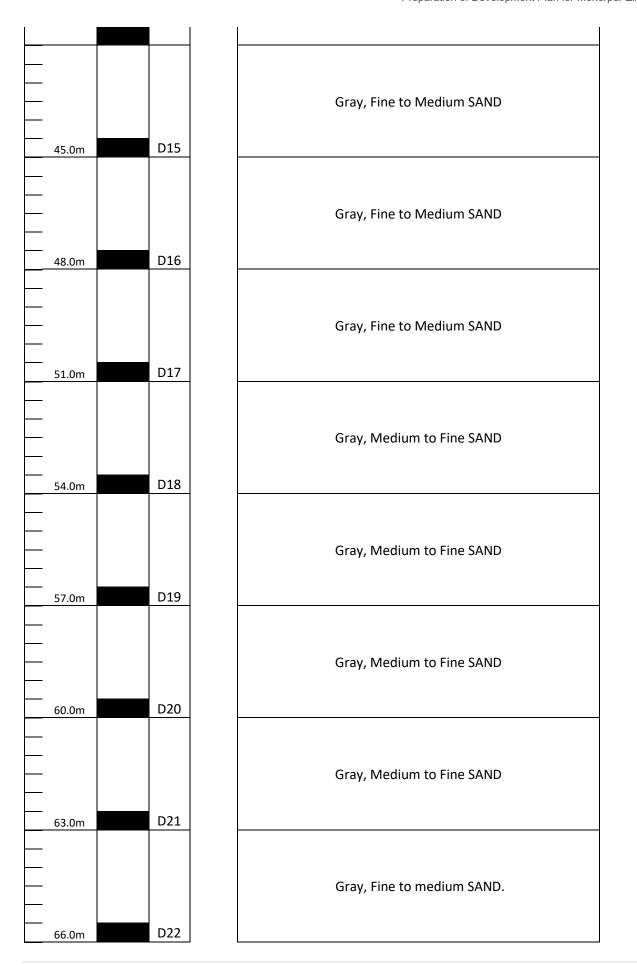
Co-ordinate: 23.620828, 88.606309

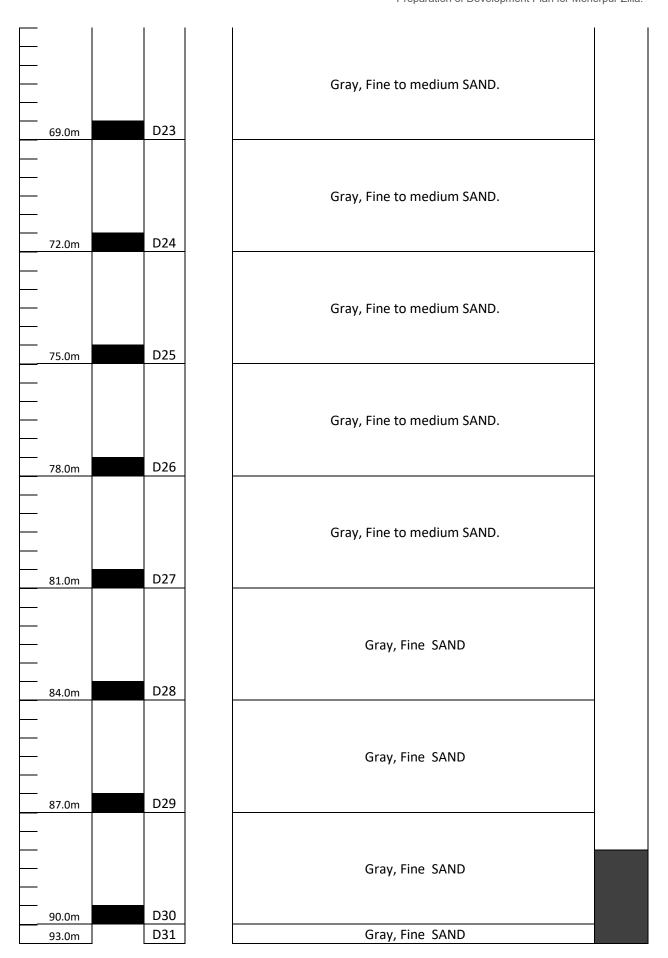
Depth of Boring: 108 Meter Ground Water Level: Meter

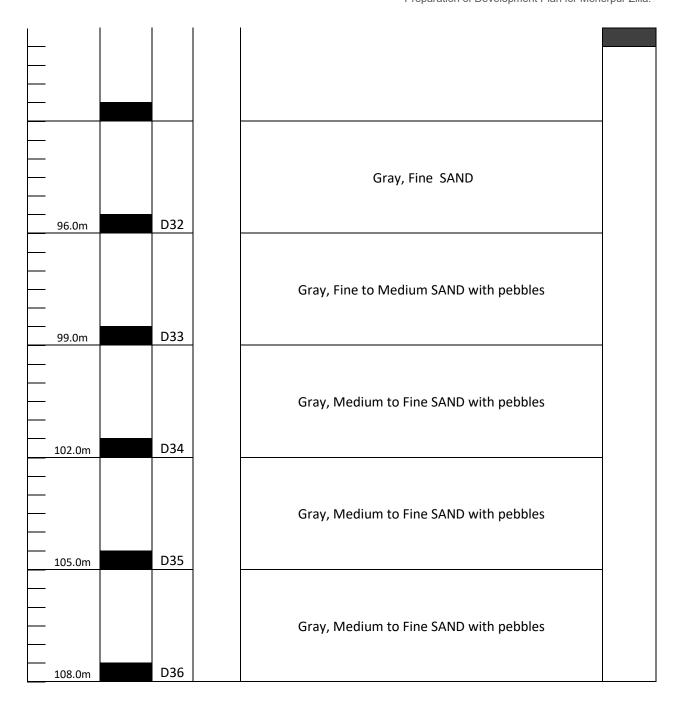
Method of Boring: Rotary Wash Boring

Method of Boring		wash Bo	ring	
Boring Diameter:				
Date: 28/02/2025				-
Depth Bellow GL (m) Type of	Sample Sample No	Thickness (m)	Lithologic Description	Layer Change
3.0m	D1		Brownish Gray, Silty CLAY	
			Brownish Gray, Silty CLAY	
6.0m	D2		Brownish Gray, Silty CLAY	
	D4		Gray, Very Fine to Fine SAND	
	D5		Gray, Very Fine to Fine SAND	
18.0m	D6		Gray, Very Fine to Fine SAND	







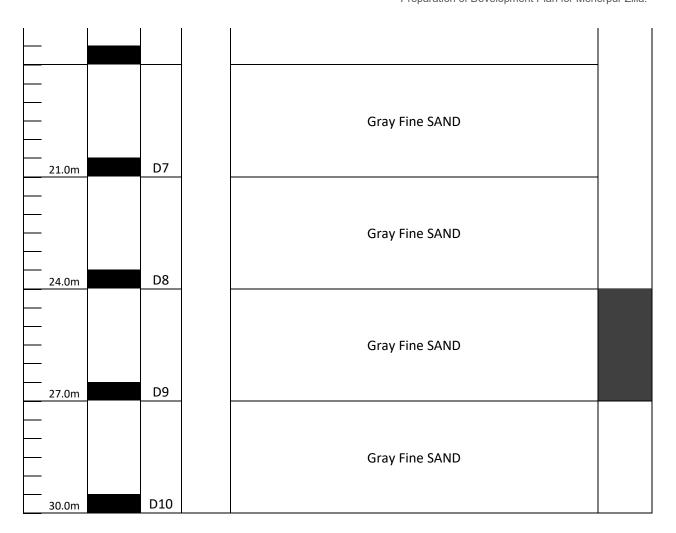


Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla

Project: Hyd	rogeological	Surveys a	and Studies under Development Plan for Meherpur Zilla				
Client: Urban Development Directorate (UDD)							
Co-ordinate: Depth of Bor	ondobas Dol 23.620828, ring: 30 Mete	88.60630 er	Govt. Primary School, Mehi 09 GEOSERVICES & RESE/	∧≳сн			
	Ground Water Level: Meter Method of Boring: Rotary Wash Boring						
Boring Diam		y wash Bo	oring				
Date: 02/03/							
		E		ge			
Depth Bellow GL (m)	Type of Sample Sample Sample No	Thickness (m	Lithologic Description	Layer Change			
3.0m	D1		Brownish Gray, Silty CLAY				
6.0m	D2		Brownish Gray, Silty CLAY				
9.0m	D3		Gray, Very Fine to Fine SAND.				
12.0m	D4		Gray, Very Fine to Fine SAND.				
	D5		Gray, Very Fine to Fine SAND.				
15.0m	D3	-	Gray Fine SAND				

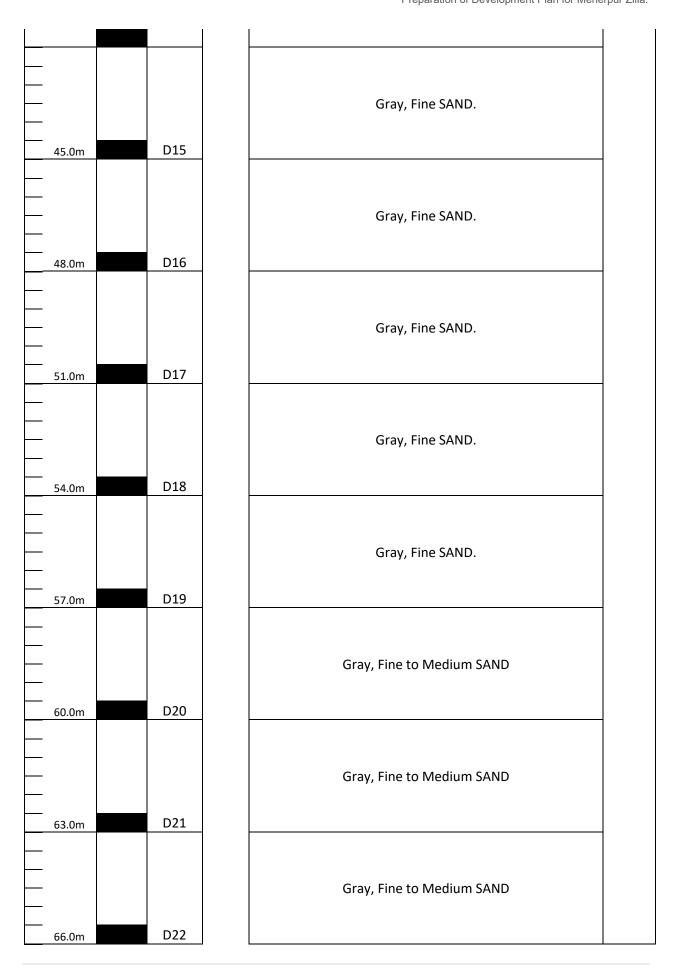
D6

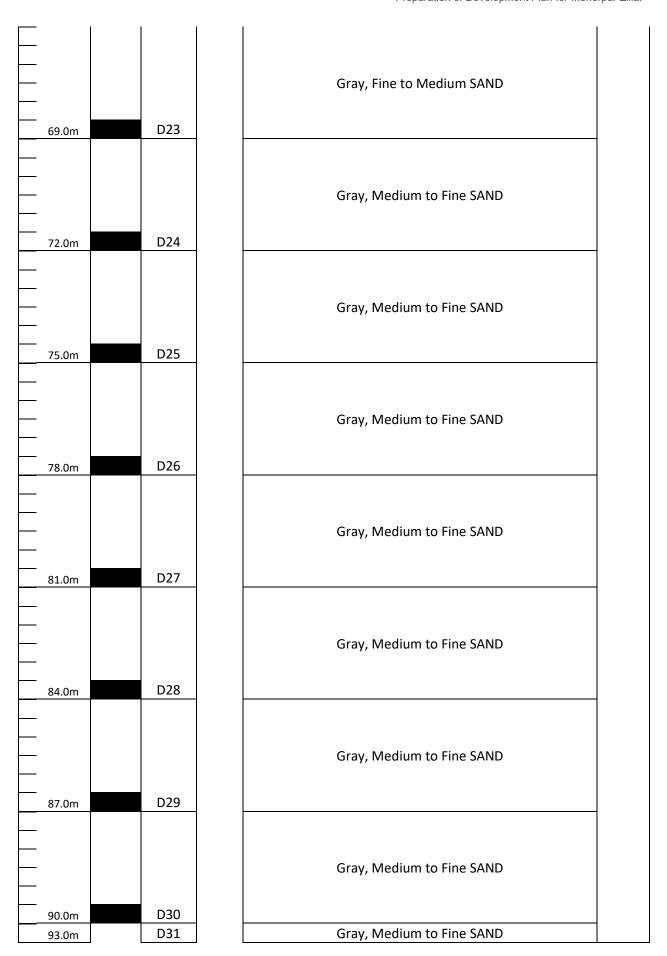
18.0m

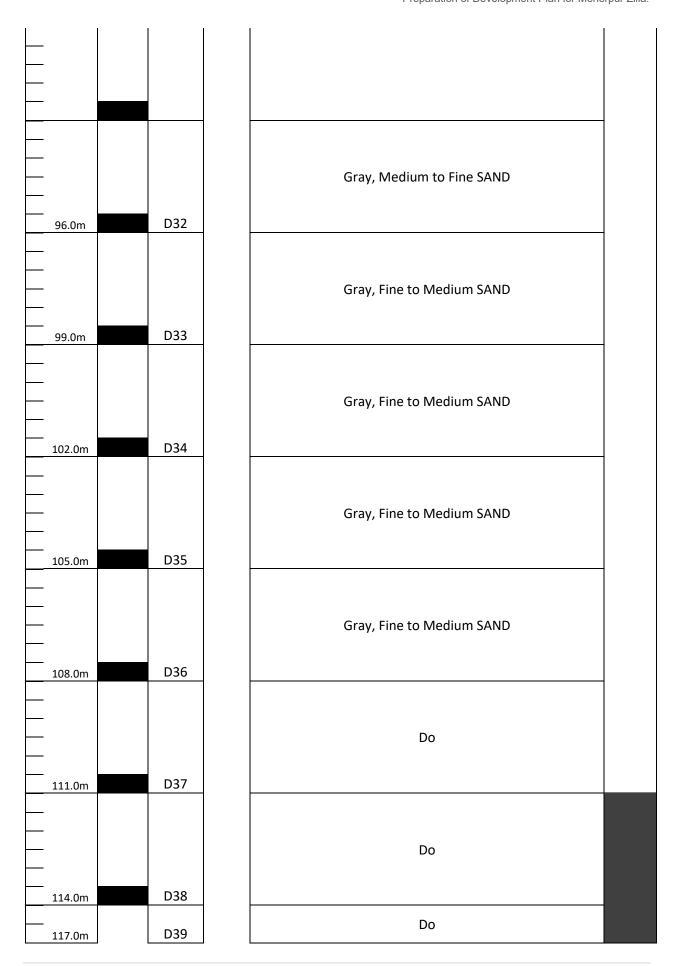


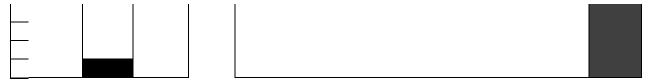
Project: Hydrogeol	ogical Surv	ys and Studies under Development Pl	an for Meherpur Zilla			
Client: Urban Deve						
Bore Hole ID: MW-		1				
Location: Ujalpur High School, Meherpur Co-ordinate: 23.815279, 88.625642						
Depth of Boring: 117 Meter						
Ground Water Leve						
Method of Boring: Boring Diameter: 1		n Boring				
Date: 06/03/2025						
Depth Bellow GL (m) Type of Sample	Sample No	Thickness (m) Lithologic De	uoituisse Layer Change			
		Littiologic De	Scription			
		Brownish Gray	, Silty CLAY			
3.0m	D1					
6.0m	D2	Brownish Gray	, Silty CLAY			
9.0m	D3	Brownish Gray	, Silty CLAY			
12.0m	D4	Brownish Gray, Very I	Fine to Fine SAND.			
15.0m	D5	Brownish Gray, Very I	Fine to Fine SAND.			
18.0m	D6	Gray, Very Fine t	o Fine SAND.			

_		
	D7	Gray, Very Fine to Fine SAND.
	D8	Gray, Very Fine to Fine SAND.
	D9	Gray, Fine SAND.
30.0m	D10	Gray, Fine SAND.
	D11	Gray, Fine SAND.
	D12	Gray, Fine SAND.
	D13	Gray, Fine SAND.
42.0m	D14	Gray, Fine SAND.

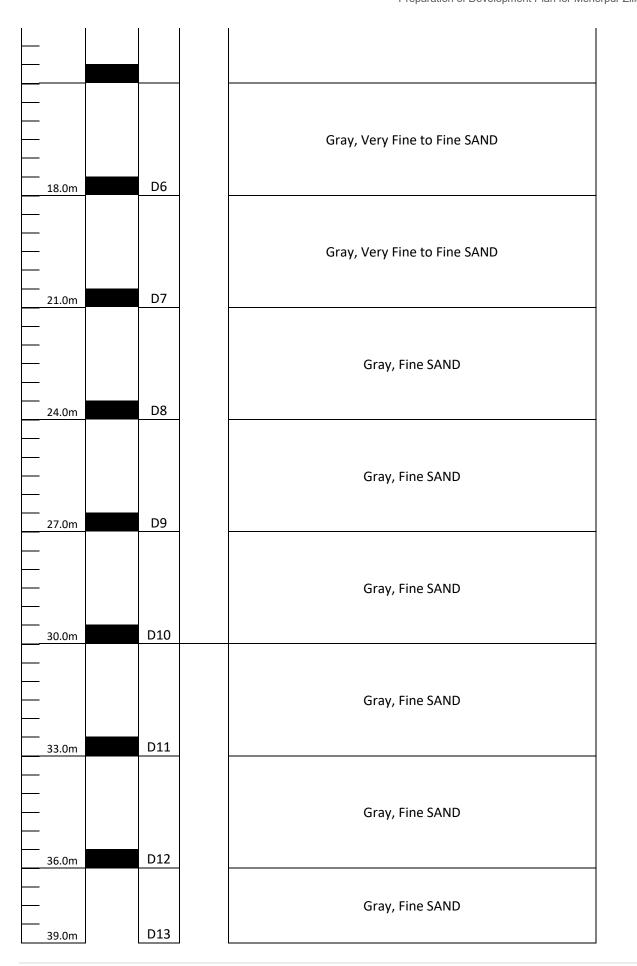


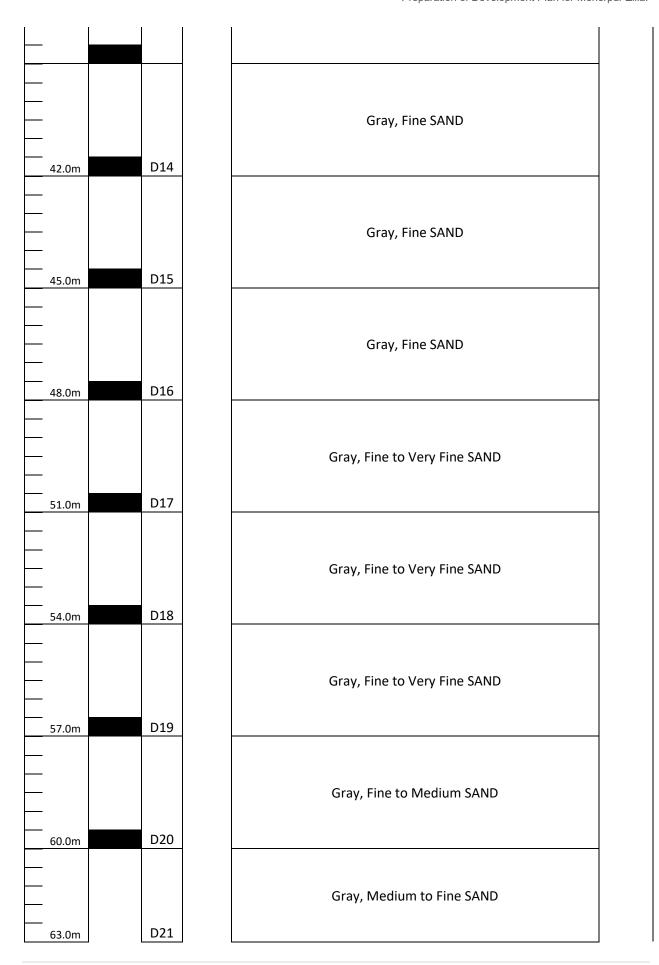


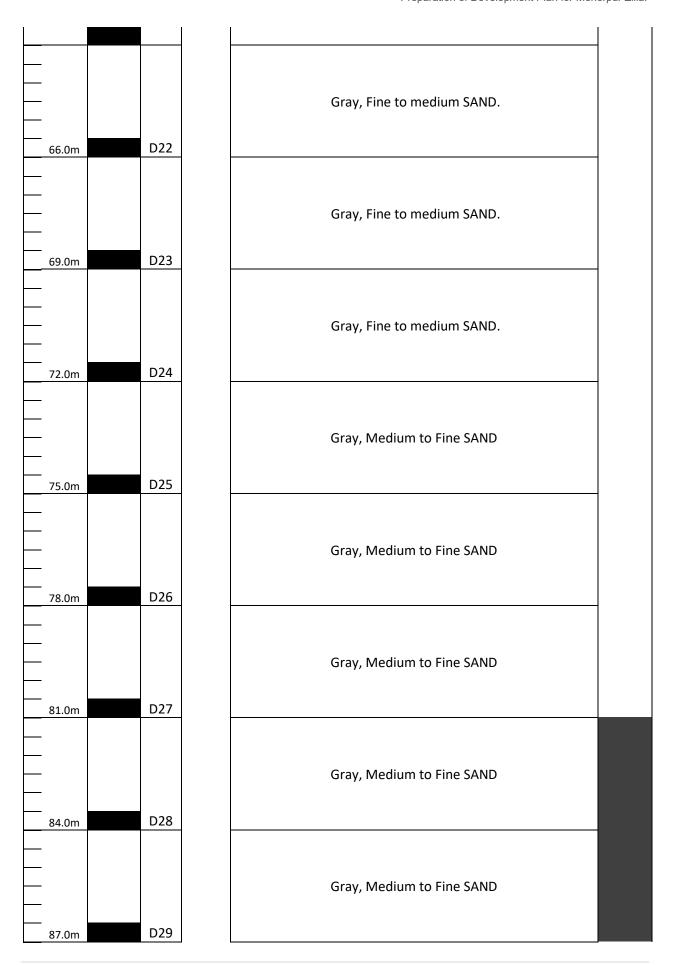


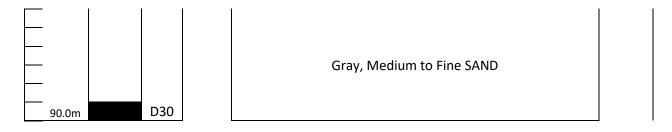


Project: H	lydrogeol	ogical	Surveys	and Studies under Development Plan for Meherpur Zilla		
Client: Ur	ban Deve	lopme	nt Direct	torate (UDD)		
				W 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1	
Bore Hole					Ц	
Location: Ujalpur High School, Meherpur						
Co-ordinate: 23.815279, 88.625642 Depth of Boring: 90 Meter						
Ground W				oring		
Method o			vvasii b	oring		
Date: 08/						
	of		Œ		ge	
ello	Type of Sample	Sample No	Thickness (m)		Layer Change	
oth Bell GL (m)	T	am	kne		er C	
Depth Bellow GL (m)		5	Thic	Lithologic Description	Lay	
_				Enthologie Description		
				Describb Con City CLAV		
				Brownish Gray, Silty CLAY		
3.0m		D1				
				Brownish Gray, Silty CLAY		
6.0m		D2				
0.0111		DZ				
				Brownish Gray, Very Fine to Fine SAND		
9.0m		D3				
<u> </u>				Gray, Very Fine to Fine SAND		
<u> </u>						
12.0m		D4				
12.0m		<i>D</i> 4				
<u> </u>				Gray, Very Fine to Fine SAND		
 15.0m		D5		2.31, 12.1 65 1 3 3		
	l l				į į	

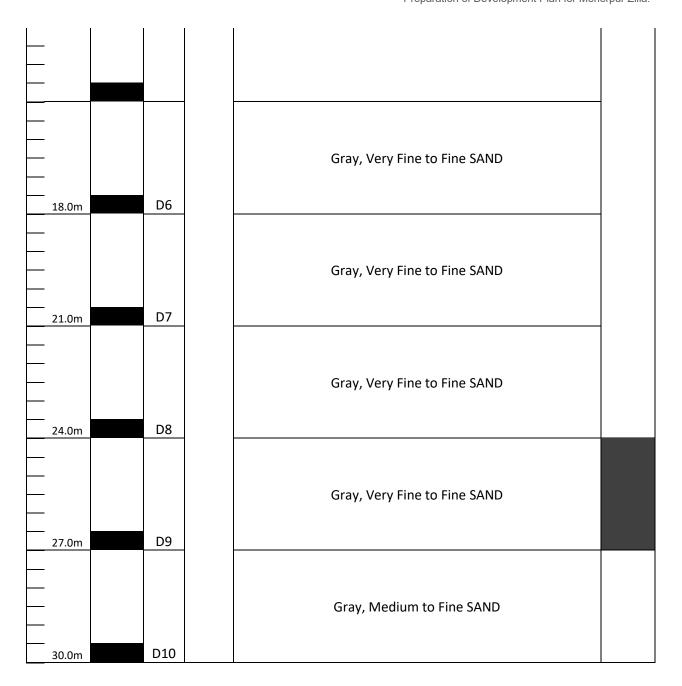






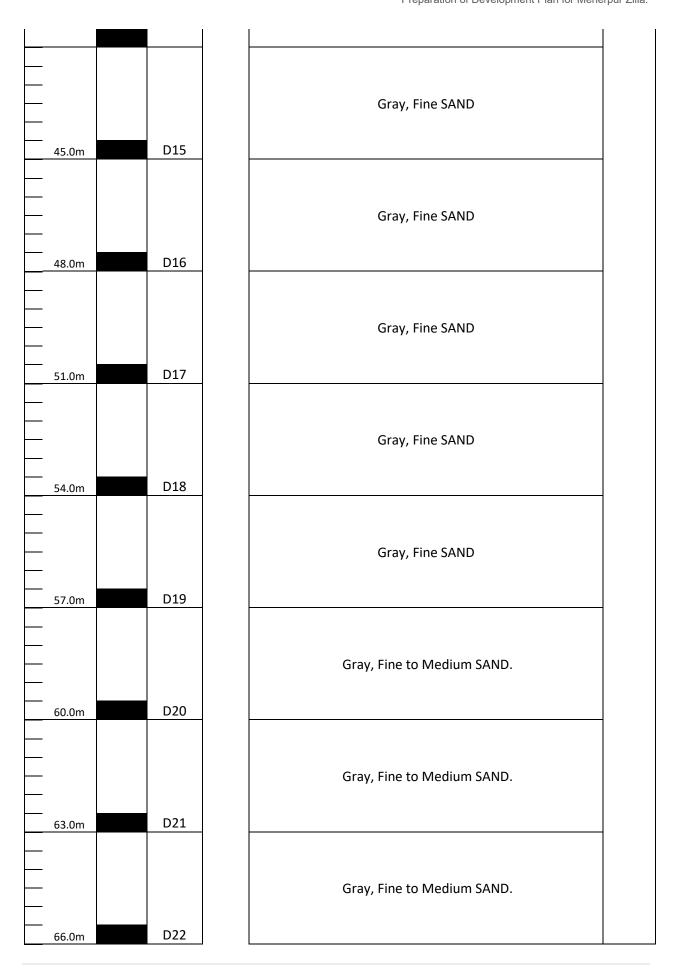


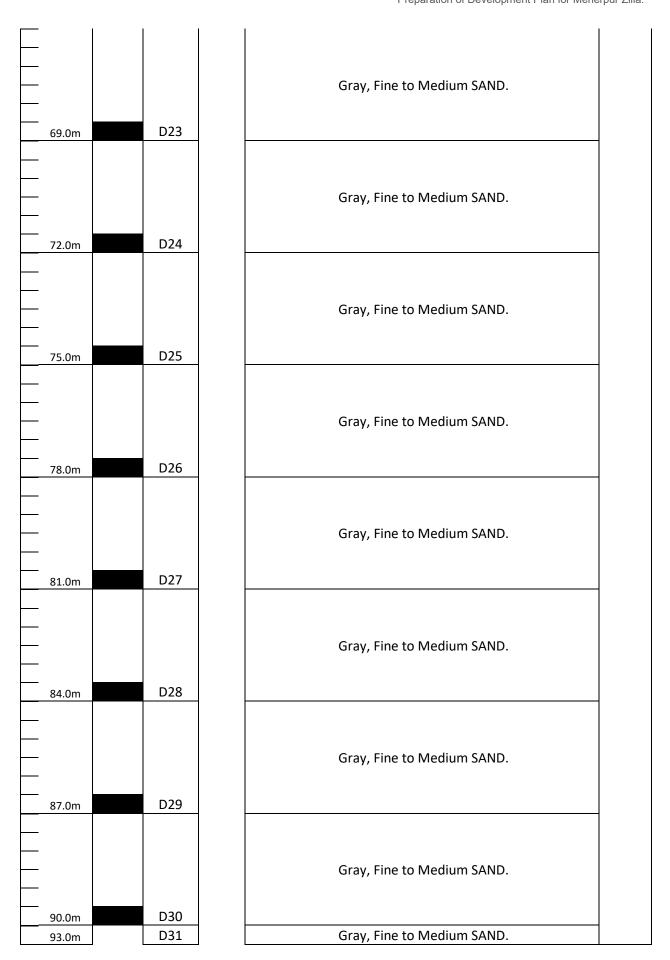
_					
				and Studies under Development Plan for Meherpur Zilla	
Client: Ur	ban Deve	lopme	nt Direct	torate (UDD)	
Bore Hole	e ID: MW	-03S			
Location:			nool, Me	eherpur	
Co-ordina		_		I CENTED CODI	VSCH.
Depth of				II.GEOSERVICES & RESER	INCH
Ground V	Vater Lev	el: Met	er		
Method o			Wash B	Boring	
Boring Di		5"			
Date: 09/	03/2025	0		T T	υ ·
Depth Bellow GL (m)	Type of Sample	Sample No	Thickness (m)	Lithologic Description	Layer Change
				1 1 0 1 1 1 1	
3.0m		D1		Brownish Gray, Silty CLAY	
6.0m		D2		Brownish Gray, Silty CLAY	
9.0m		D3		Brownish Gray, Silty CLAY	
12.0m		D4		Brownish Gray, Silty CLAY	
 15.0m		D5		Brownish Gray, Silty CLAY	

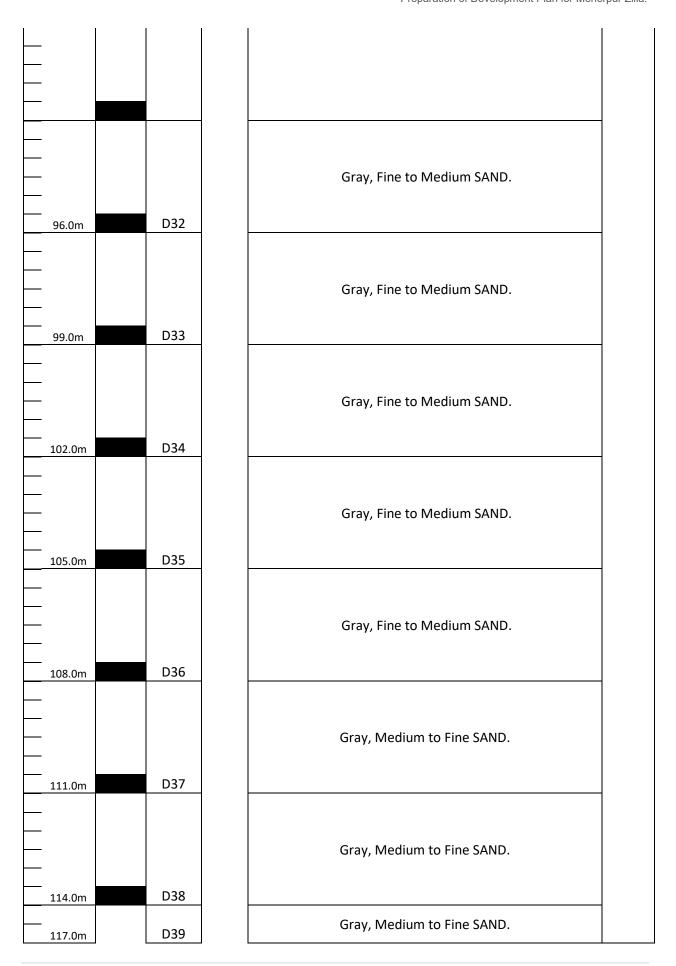


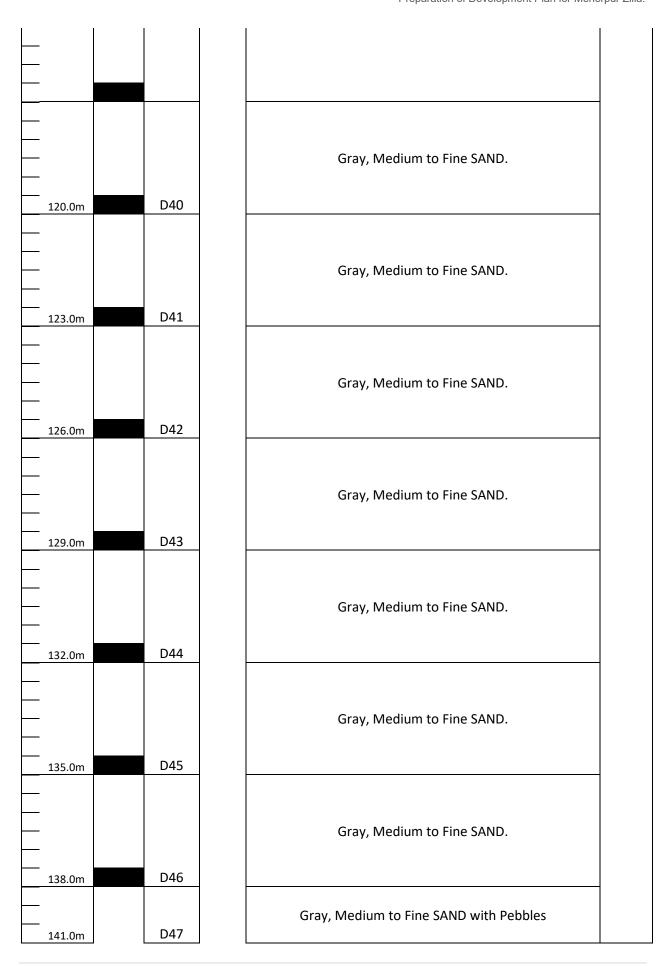
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-04D Location: Mominpur Govt. Primary School, Meherpur Co-ordinate: 23.722818,88.733614 GEOSERVICES & RESEARCH Depth of Boring: 150 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 02/03/2025 Depth Bellow GL (m) Type of Sample Layer Change Sample No Thickness (m Lithologic Description Brownish Gray, Very Fine to Fine SAND D1 3.0m Brownish Gray, Very Fine to Fine SAND D2 6.0m Brownish Gray, Very Fine to Fine SAND D3 9.0m Gray, Very Fine to Fine SAND D4 12.0m Gray, Very Fine to Fine SAND D5 15.0m Gray, Fine to Very Fine SAND D6 18.0m

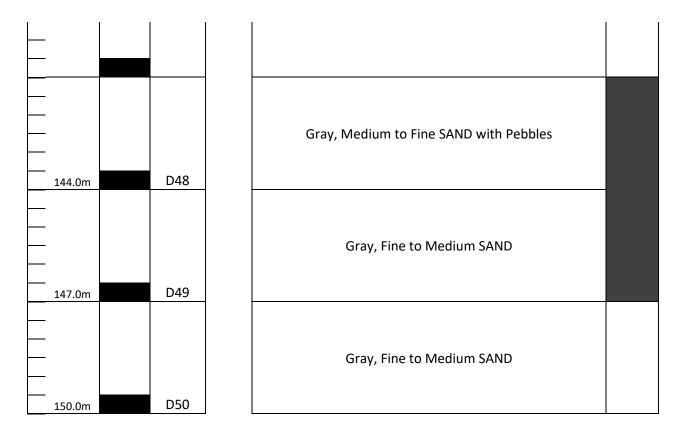
 	I		
_			0 5
			Gray, Fine to Very Fine SAND
21.0***		D7	
21.0m		<i>D7</i>	
_			Gray, Fine to Very Fine SAND
24.0m		D8	
<u> </u>			Cray Fine to Very Fine SAND
 			Gray, Fine to Very Fine SAND
 27.0m		D9	
27.0111			
_			
			Gray, Fine SAND
30.0m		010	
_			
			Gray, Fine SAND
			Gray, Title SAND
 33.0m		011	
			Gray, Fine SAND
<u> </u>			
36.0m		012	
<u> </u>			Gray, Fine SAND
			Gray, Tille Salvo
 39.0m		013	
_			Gray Fina SAND
			Gray, Fine SAND
42.0m		D14	



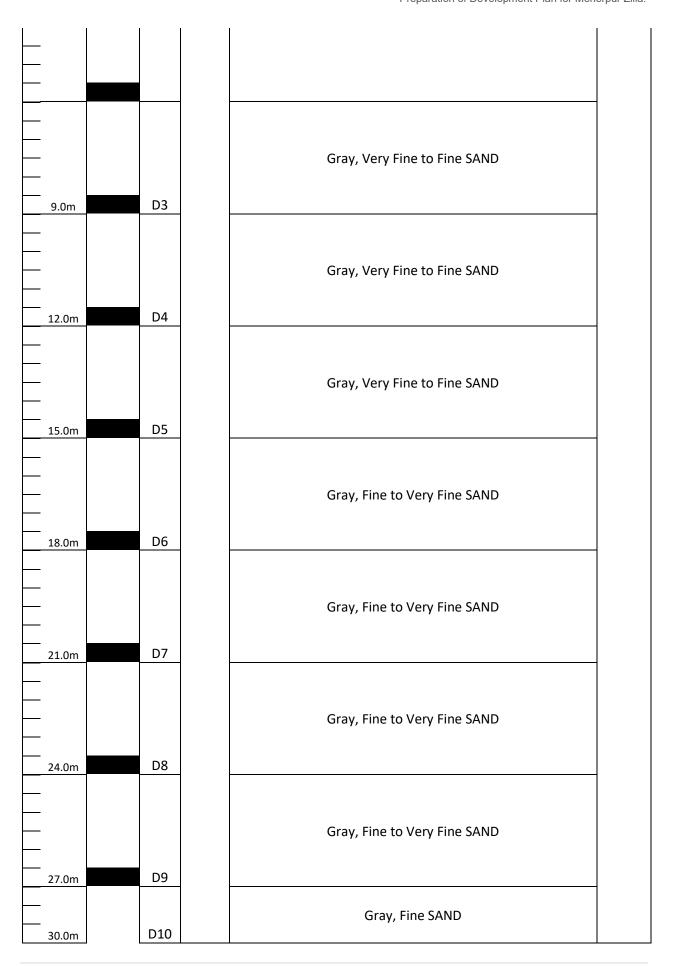


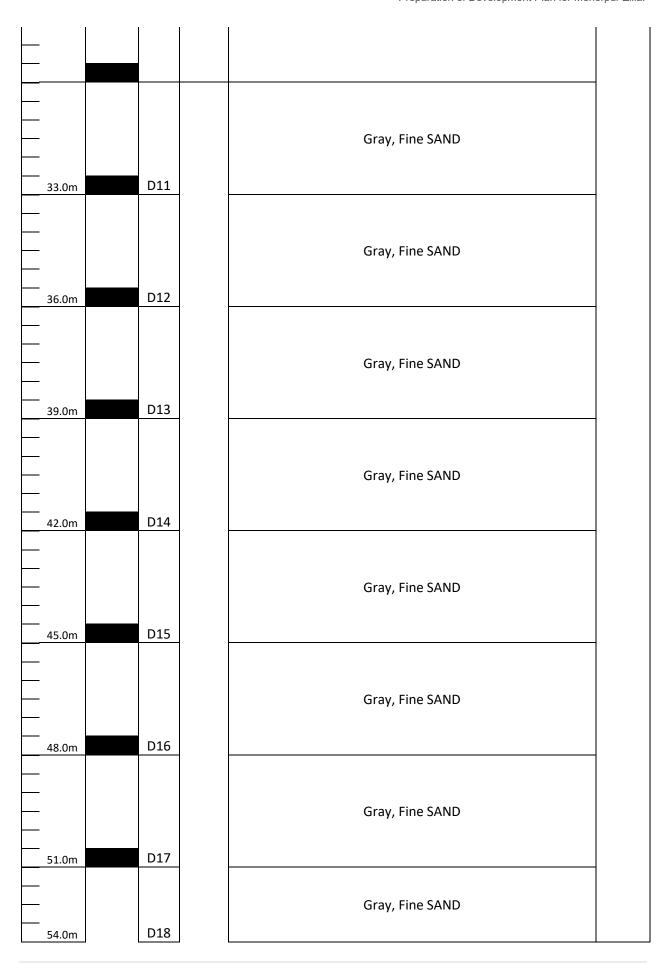


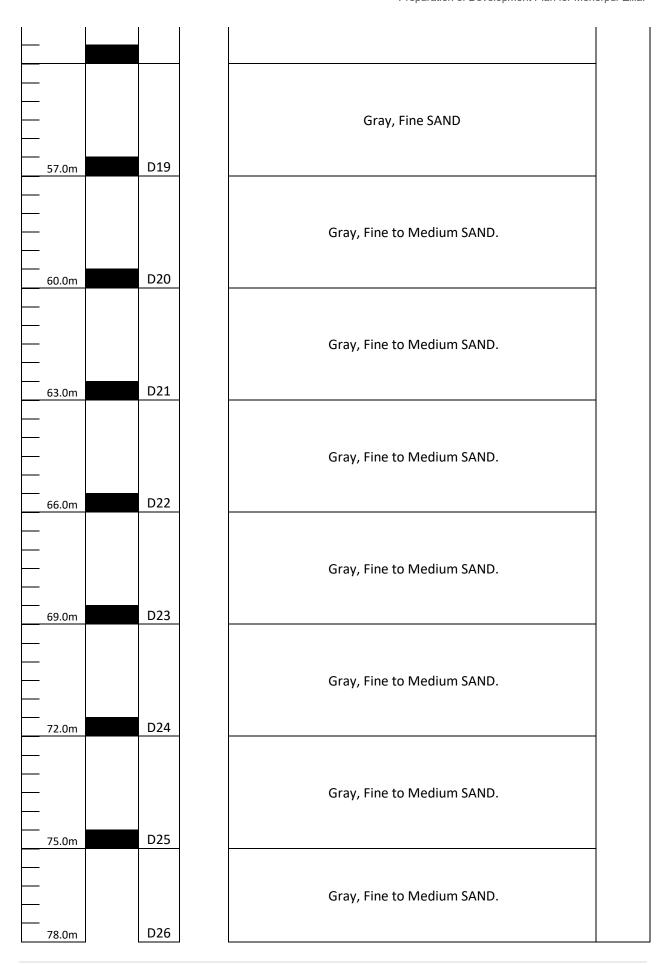


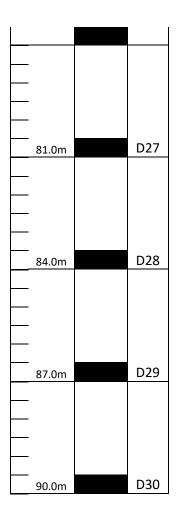


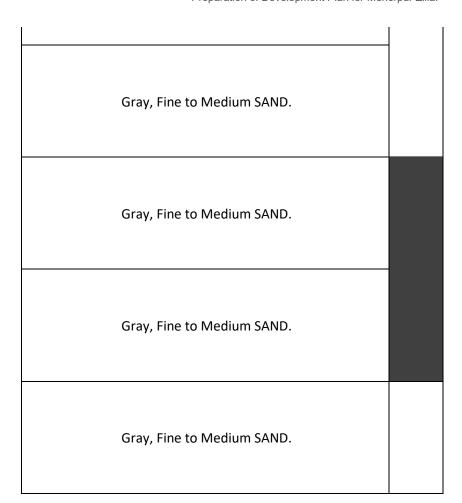
Project: H	lydrogeol	ogical S	Surveys a	and Studies under Develop	ment Plan for Meherpur Zilla					
Client: Ur	ban Deve	lopme	nt Direct	orate (UDD)						
Bore Hole ID: MW-04I										
Location: Mominpur Govt. Primary School, Meherpur Co-ordinate: 23.722818,88.733614										
				4	GEOSERVICES & RESE/	/SCH				
Depth of I						1/2010/1000				
Ground W										
Method o			Wash B	oring						
Boring Dia		5"								
Date: 04/0						4)				
Depth Bellow GL (m)	Type of Sample	Sample No	Thickness (m)			Layer Change				
oth Bell GL (m)	Тур	ldu	ıes			Chi				
pth GL		Sar	ickı			yer				
De			보	Litholo	gic Description	La				
				Drownish Crov	Vary Fine to Fine CAND					
				Brownish Gray,	, Very Fine to Fine SAND					
3.0m		D1								
6.0m		D2		Brownish Gray,	, Very Fine to Fine SAND					











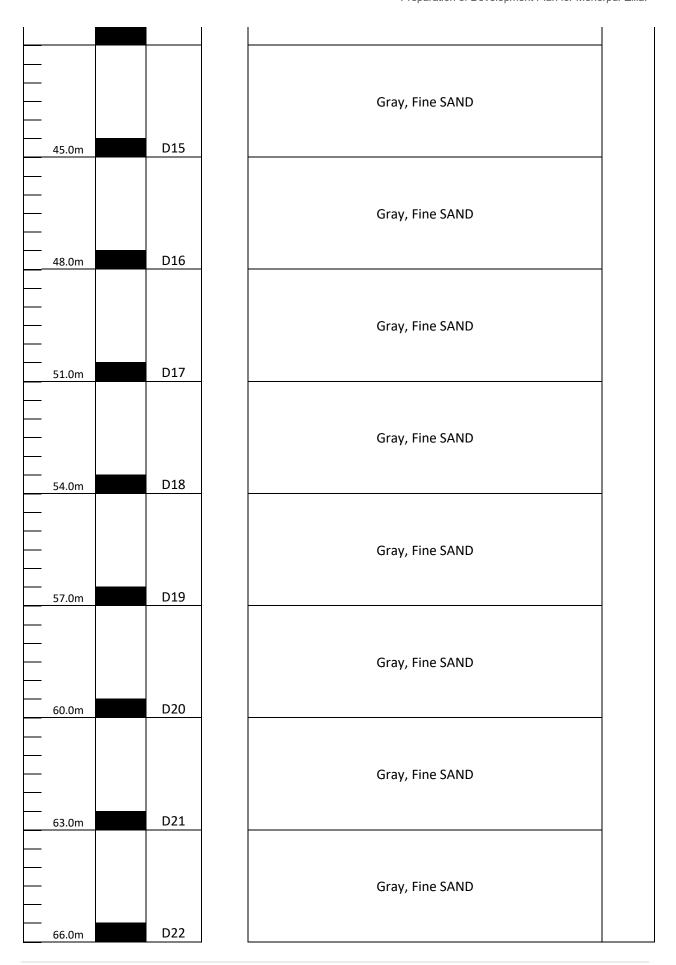
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-04S Location: Mominpur Govt. Primary School, Meherpur Co-ordinate: 23.722818,88.733614 GEOSERVICES & RESEARCH Depth of Boring: 30 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 05/03/2025 Type of Sample Layer Change Sample No Thickness (m) Depth Bellow GL (m) Lithologic Description Brownish Gray, Very Fine to Fine SAND D1 3.0m Brownish Gray, Very Fine to Fine SAND 6.0m D2 Brownish Gray, Very Fine to Fine SAND D3 9.0m Brownish Gray, Very Fine to Fine SAND D4 12.0m Gray, Very Fine to Fine SAND D5 15.0m Gray, Very Fine to Fine SAND D6 18.0m

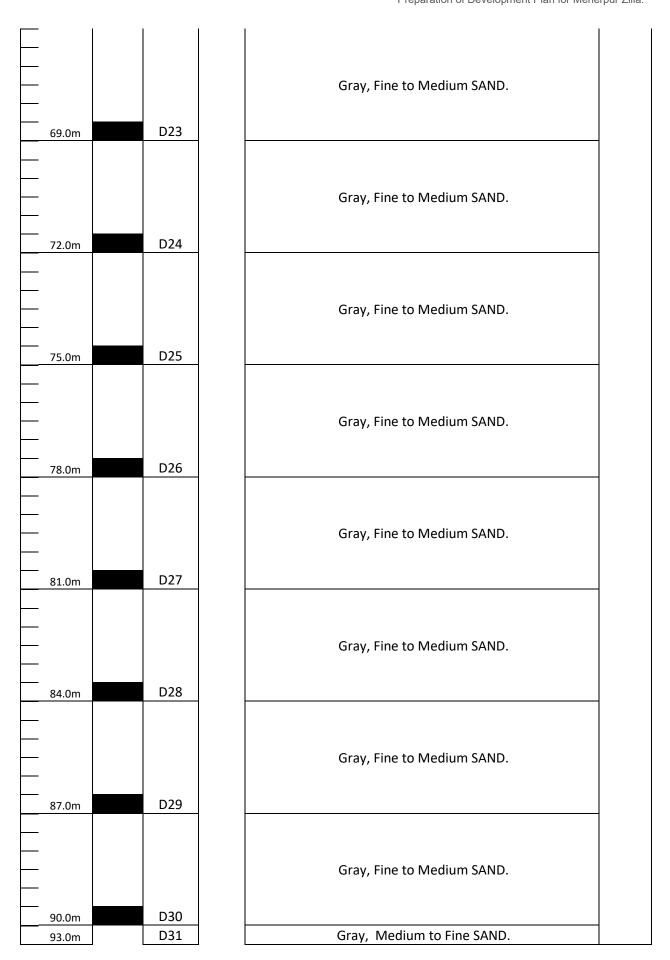
	D7	Gray, Very Fine to Fine SAND	
	D8	Gray, Fine SAND	
	D9	Gray, Fine SAND	
30.0m	D10	Gray, Fine SAND	

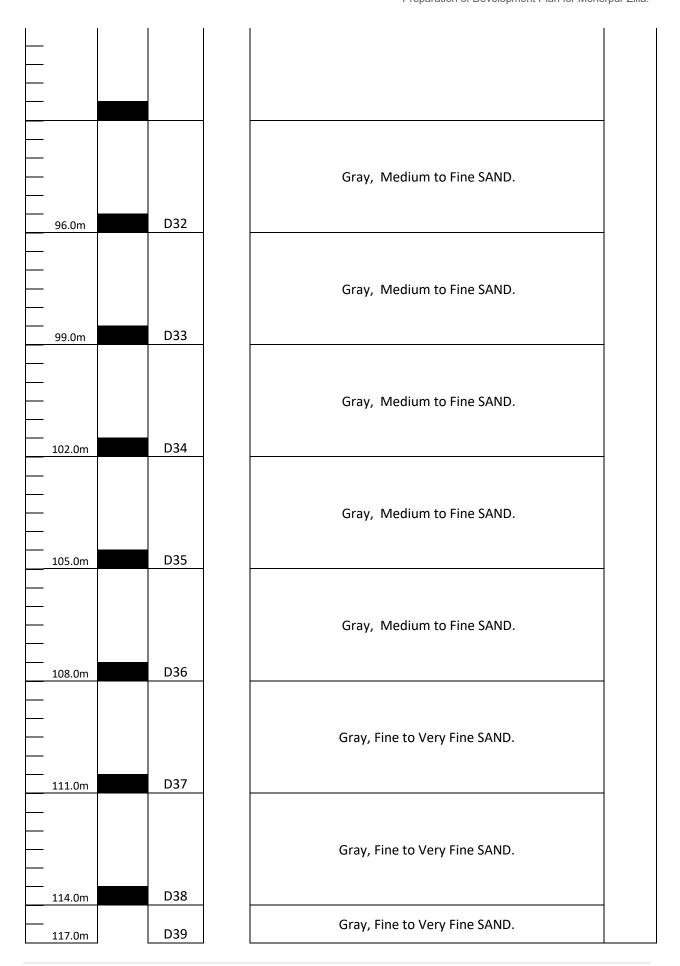
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla
Client: Urban Development Directorate (UDD)

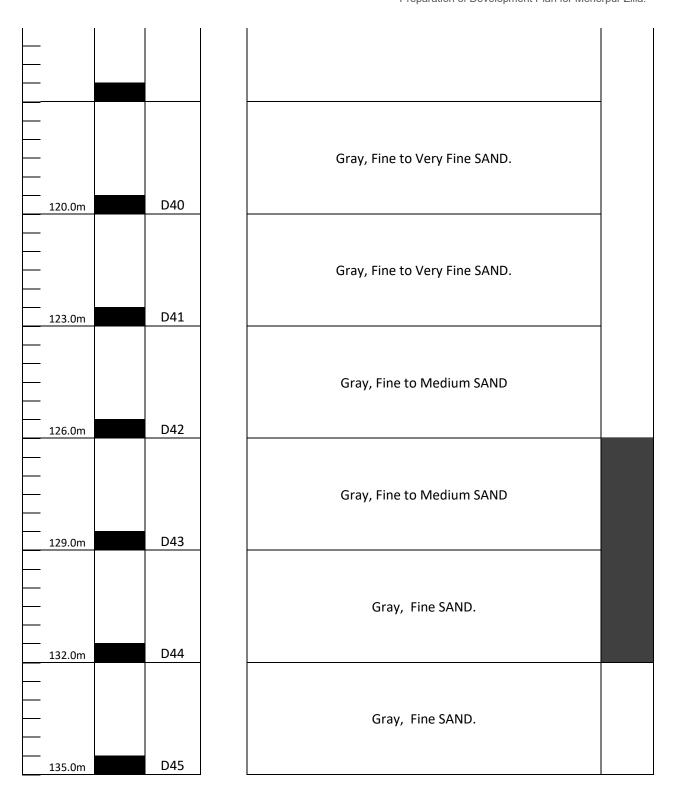
Client:	Urban Deve	elopment [Director	ate (UDD)						
Bore Hole ID: MW-05D Location: Bashbaria High School, Gangni, Meherpur Co-ordinate: 23.810792,88.732036 Depth of Boring: 135 Meter										
	l Water Lev									
Method	d of Boring:	Rotary Wa	ash Bori	ng						
	Diameter: 1									
	0/03/2025									
Denth Bellow	<u> </u>	Sample No	Thickness (m)	Lithologic Description	Layer Change					
3.00	n	D1		Brownish Gray, Silty CLAY						
 	n	D2		Brownish Gray, Silty CLAY						
9.01	m	D3		Brownish Gray, Very Fine to Fine SAND						
12.0	ım.	D4		Brownish Gray, Very Fine to Fine SAND						
15.0		D5		Gray, Very Fine to Fine SAND						
18.0		D6		Gray, Very Fine to Fine SAND						

<u> </u>		
	D7	Gray, Very Fine to Fine SAND
	D8	Gray, Very Fine to Fine SAND
	D9	Gray, Fine SAND
30.0m	D10	Gray, Fine SAND
	D11	Gray, Fine SAND
36.0m	D12	Gray, Fine to Medium SAND
39.0m	D13	Gray, Fine to Medium SAND
42.0m	D14	Gray, Fine SAND



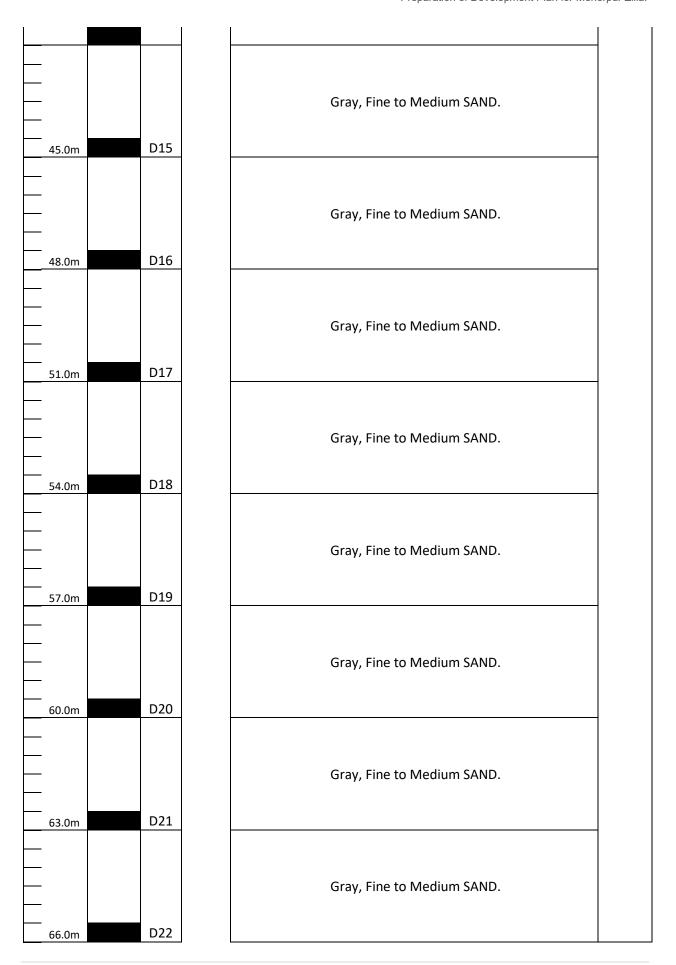


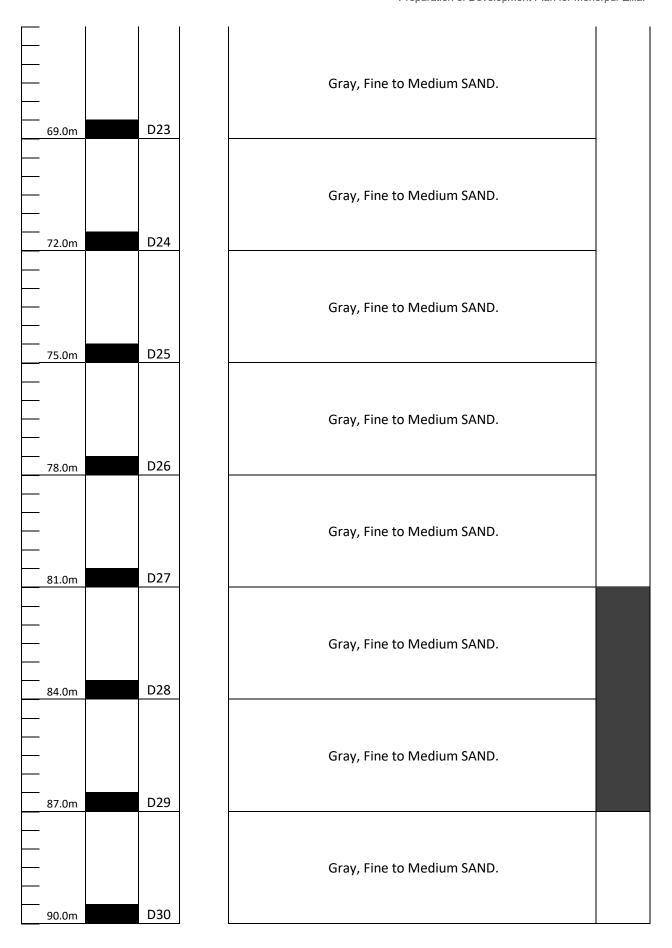




Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-05I Location: Bashbaria High School, Gangni, Meherpur Co-ordinate: 23.810792,88.732036 IGEOSERVICES & RESEARCH Depth of Boring: 90 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 12/03/2025 Type of Sample Depth Bellow GL (m) Layer Change Sample No Thickness (m Lithologic Description Brownish Gray, Silty CLAY 3.0m D1 Brownish Gray, Very Fine to Fine SAND D2 6.0m Brownish Gray, Very Fine to Fine SAND D3 9.0m Gray, Very Fine to Fine SAND D4 12.0m Gray, Very Fine to Fine SAND D5 15.0m Gray, Very Fine to Fine SAND D6 18.0m

		Gray, Very Fine to Fine SAND
		, ,
21.0m	D7	
22.011		
		Gray, Fine SAND
		,,
24.0m	D8	
24.0111		
		Gray, Fine SAND
		5.57, 5
27.0m	D9	
27.0111		
		Gray, Fine SAND
		Gray, Time 37 (14)
30.0m	D10	
30.0m	D10	
_		Gray, Fine SAND
		Gray, Time 5, and
33.0m	D11	
33.0111	D11	
		Gray, Fine SAND
		5.57, 5
36.0m	D12	
30.0111	512	
		Gray, Fine SAND
		Stay, The Stay
20.0m	D13	
39.0m	213	
_		Gray, Fine SAND
42.0m	D14	





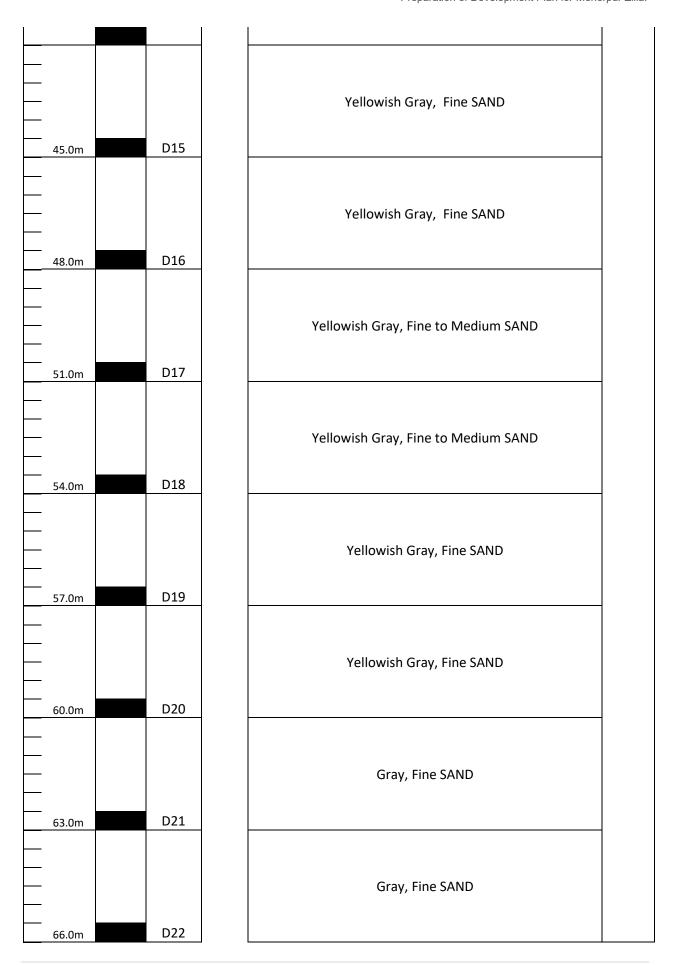
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-05S Location: Bashbaria High School, Gangni, Meherpur Co-ordinate: 23.810792,88.732036 GEOSERVICES & RESEARCH Depth of Boring: 30 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 13/03/2025 Type of Sample Depth Bellow GL (m) Layer Change Sample No Thickness (m) Lithologic Description Brownish Gray, Silty CLAY 3.0m D1 Gray, Very Fine to Fine SAND 6.0m D2 Gray, Very Fine to Fine SAND D3 9.0m Gray, Very Fine to Fine SAND D4 12.0m Gray, Very Fine to Fine SAND D5 15.0m Gray, Very Fine to Fine SAND D6 18.0m

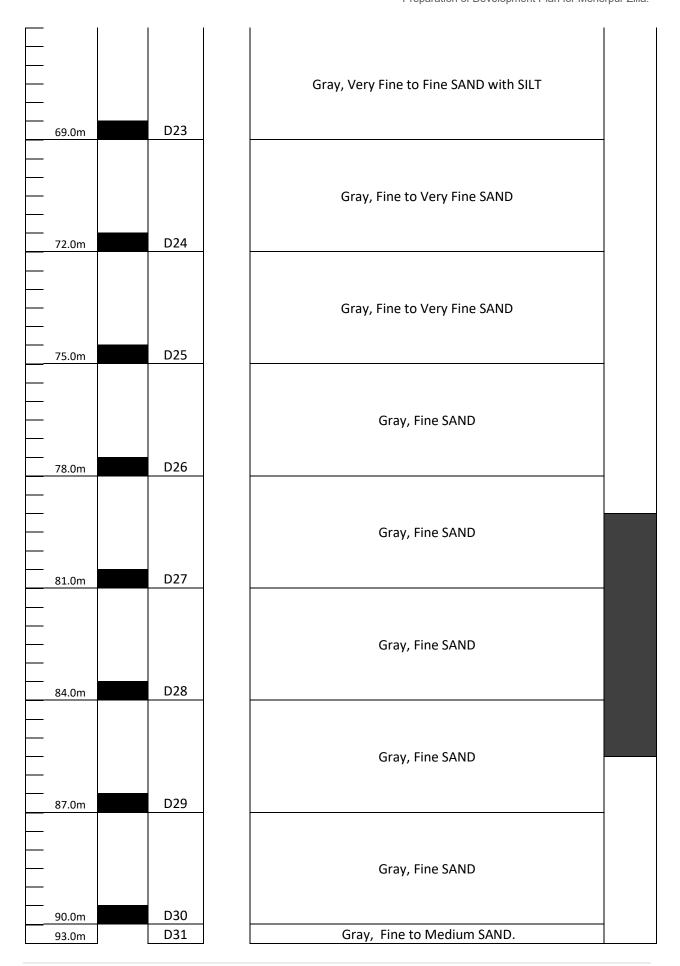
Gray, Fine SAND 21.0m D7 Gray, Fine SAND Gray, Fine SAND		
21.0m D7 Gray, Fine SAND		
Gray, Fine SAND		D7
	_ _ _	
	24.0m	D8
Gray, Fine SAND 27.0m D9		D9
Gray, Fine SAND D10		D10

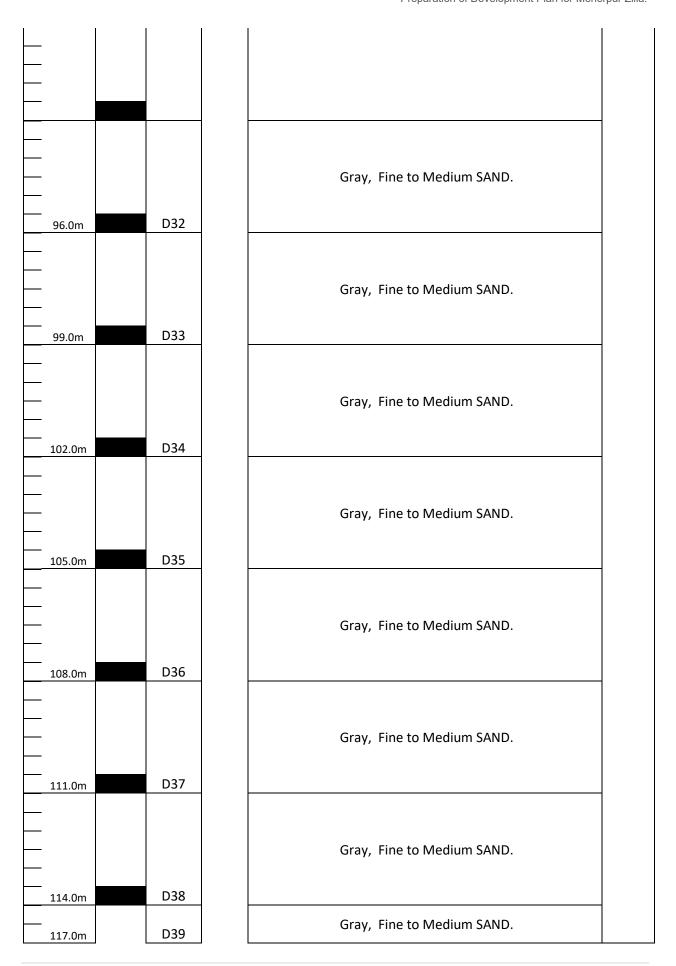
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla

	n Development		ate (UDD)	
Co-ordinate Depth of Bo	aniapukur Govt. e: 23.835775,88.7 pring: 117 Meter		School, Gangni, Meherpur GEOSERVICES & RESE	EVSCH
	ter Level: Meter			
Boring Diam	Boring: Rotary W	ash Borii	ng	
Date: 14/03				
Depth Bellow GL (m)	Type of Sample	Thickness (m)	Lithologic Description	Layer Change
	D1		Brownish Gray, Silty CLAY	
5.0m	D2		Brownish Gray, Silty CLAY	
9.0m	D3		Gray, Silty CLAY	
12.0m	D4		Gray, Silty CLAY	
	D5		Gray, Very Fine to Fine SAND with SILT	
18.0m	D6	-	Gray, Very Fine to Fine SAND with SILT	

		ì	
<u></u>			
			Gray, Silty CLAY
21.0m		D7	
			Gray, Silty CLAY
			,, ,
 24.0m		D8	
24.0111			
<u> </u>			Gray, Very Fine to Fine SAND with SILT
			S.S., T.S., T.M. CO. T.M.C. S. AND WICH SIE!
27.0		D9	
27.0m		D3	
			Vallowish Gray, Vary Fine to Fine SAND
			Yellowish Gray, Very Fine to Fine SAND
		D10	
30.0m		D10	
			Vollowish Crow Fine CAND
			Yellowish Gray, Fine SAND
<u></u>		544	
33.0m		D11	
			Vollaviah Carry Fine CAND
			Yellowish Gray, Fine SAND
36.0m		D12	
<u> </u>			V.II. 11.0 51.5115
			Yellowish Gray, Fine SAND
<u> </u>			
39.0m		D13	
<u> </u>			
<u> </u>			Yellowish Gray, Fine SAND
42.0m]	D14	

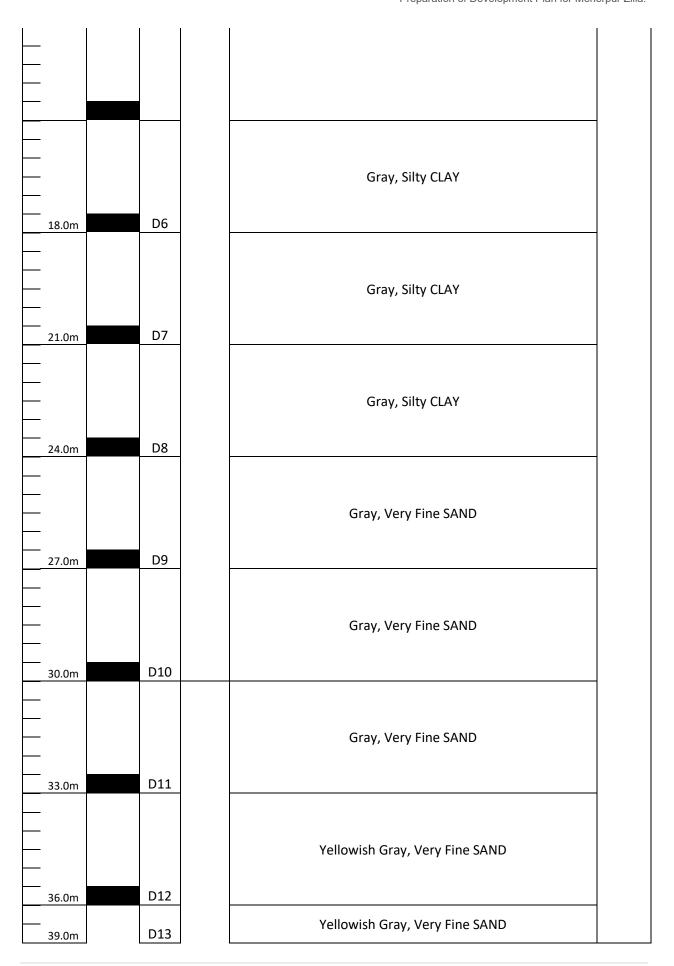


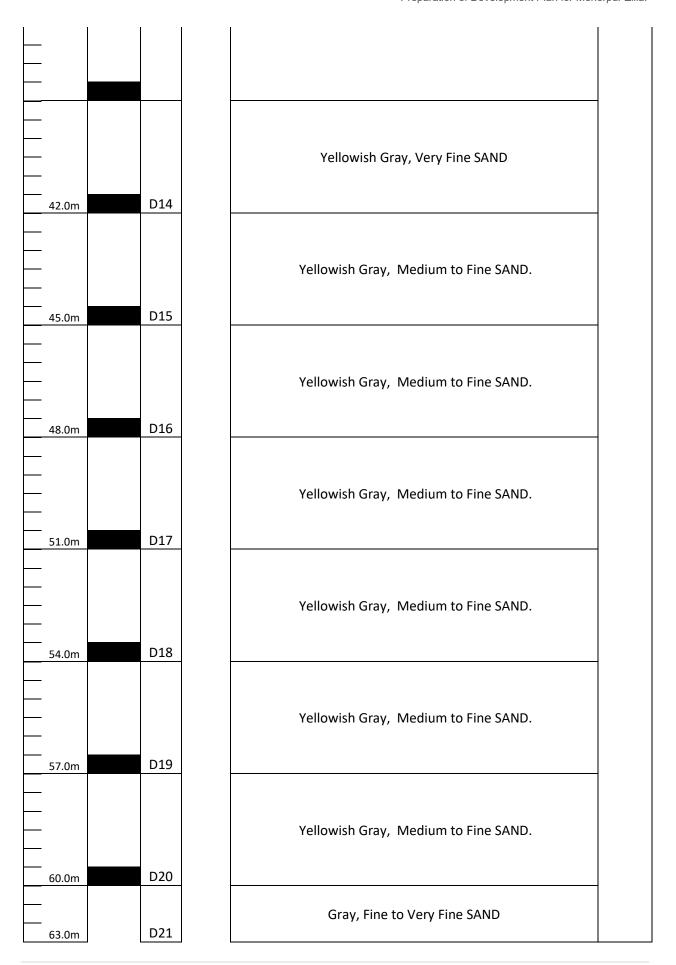


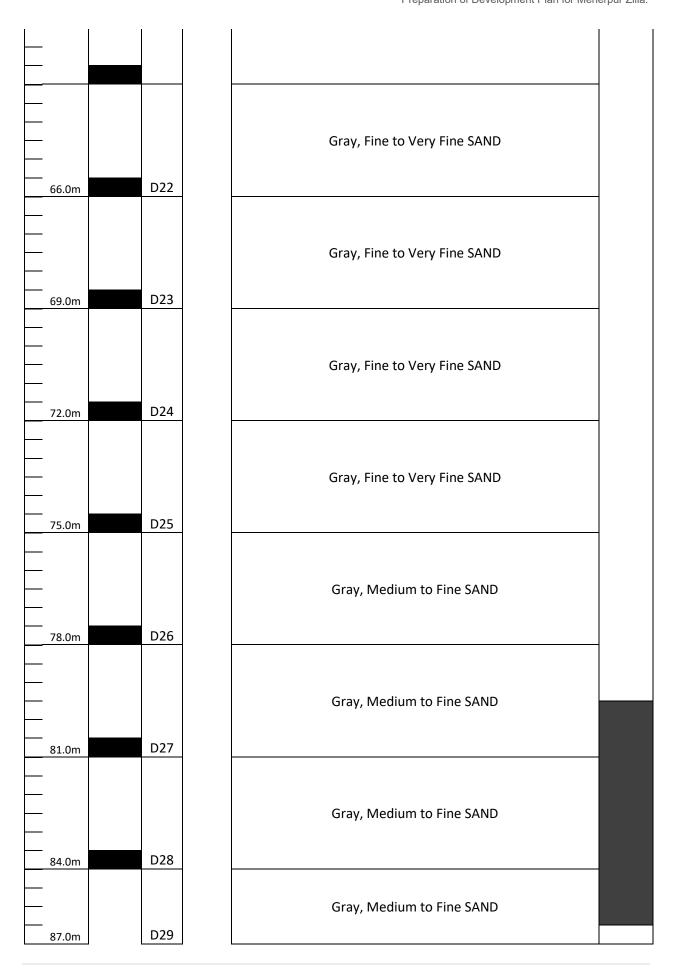




Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-061 Location: Baniapukur Govt. Primary School, Gangni, Meherpu Co-ordinate: 23.835775,88.794261 GEOSERVICES & RESEARCH Depth of Boring: 90 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 17/03/2025 Type of Sample Layer Change Sample No Depth Bellow Thickness (m GL (m) Lithologic Description Brownish Gray, Silty CLAY D1 3.0m Brownish Gray, Silty CLAY D2 6.0m Gray, Silty CLAY D3 9.0m Gray, Silty CLAY D4 12.0m D5 Gray, Silty CLAY 15.0m

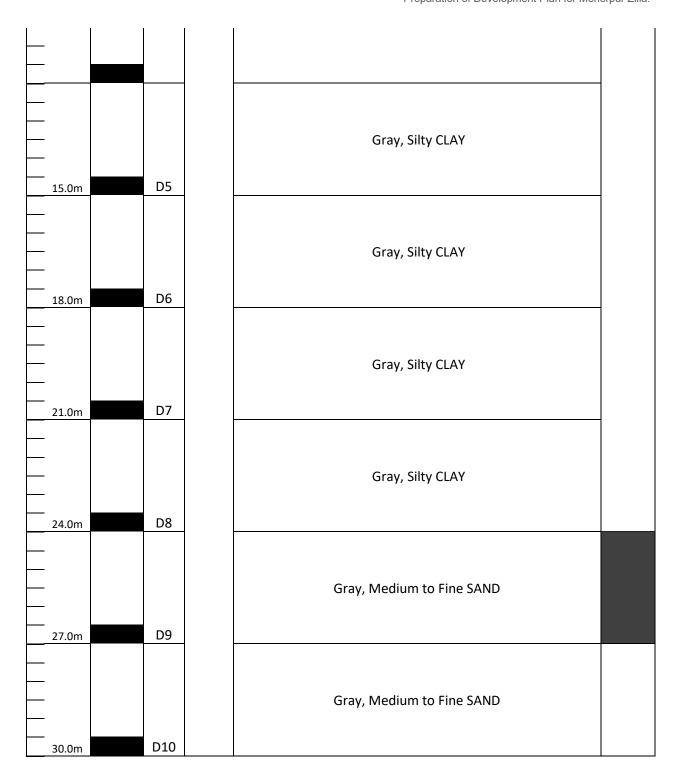




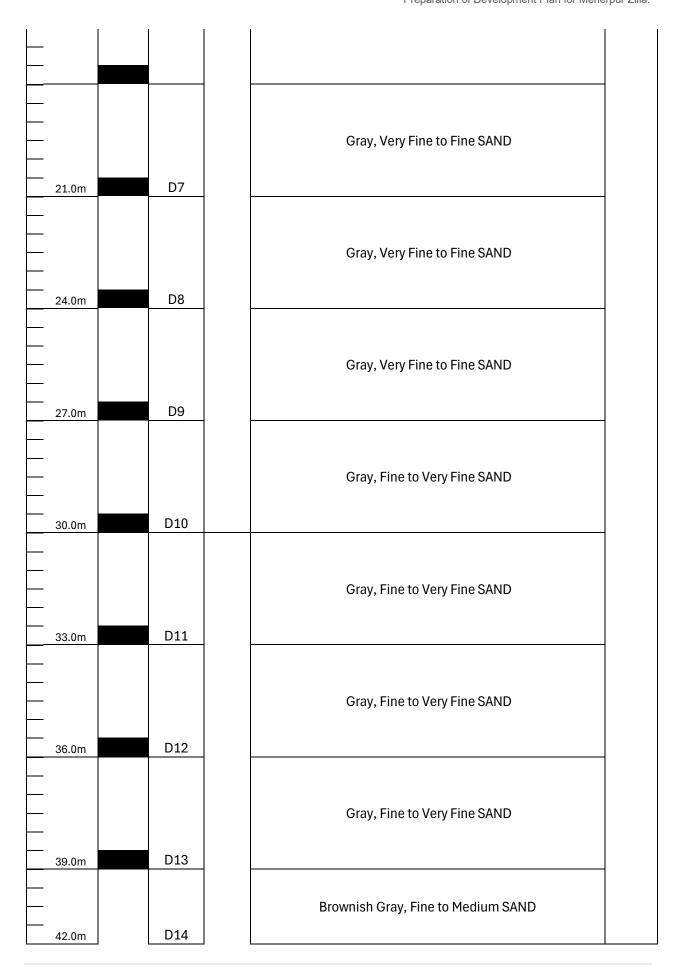


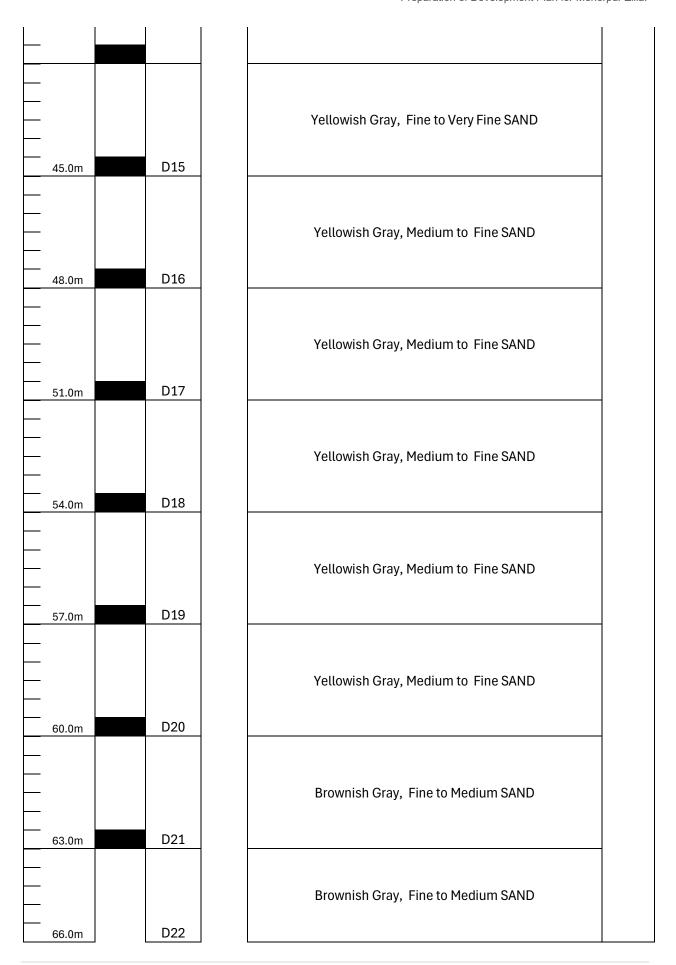


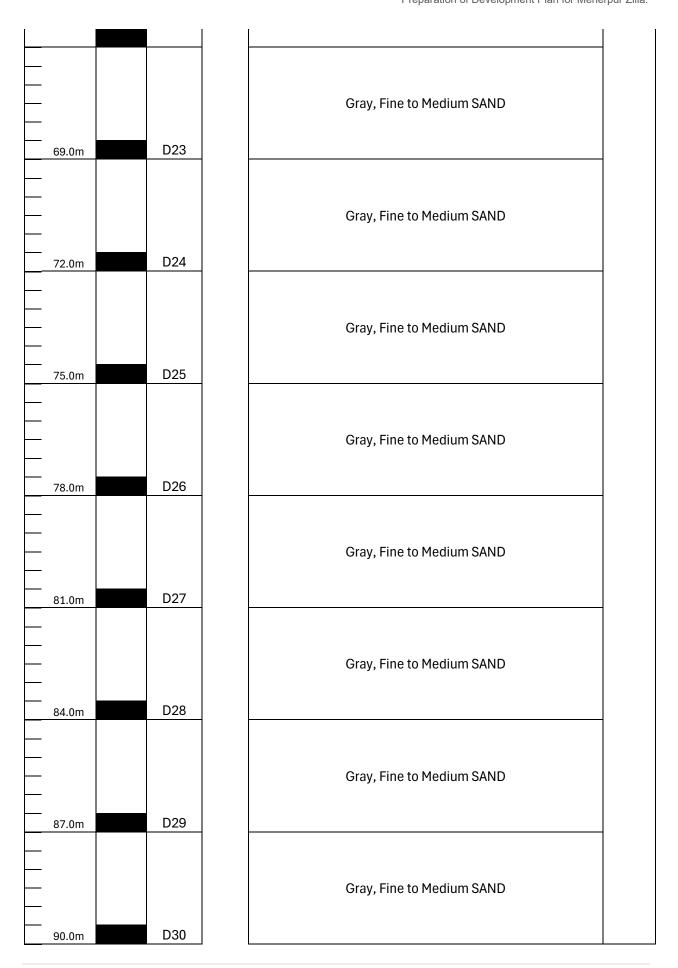
Project: Hydrogeological Surveys and Studies under Development Plan for Meherpur Zilla Client: Urban Development Directorate (UDD) Bore Hole ID: MW-06S Location: Baniapukur Govt. Primary School, Gangni, Meherpu Co-ordinate: 23.835775,88.794261 GEOSERVICES & RESEARCH Depth of Boring: 30 Meter Ground Water Level: Meter Method of Boring: Rotary Wash Boring Boring Diameter: 1.5" Date: 18/03/2025 Type of Sample Layer Change Sample No Thickness (m) Depth Bellow GL (m) Lithologic Description Brownish Gray, Silty CLAY 3.0m D1 Brownish Gray, Silty CLAY D2 6.0m Brownish Gray, Silty CLAY D3 9.0m Gray, Silty CLAY D4 12.0m

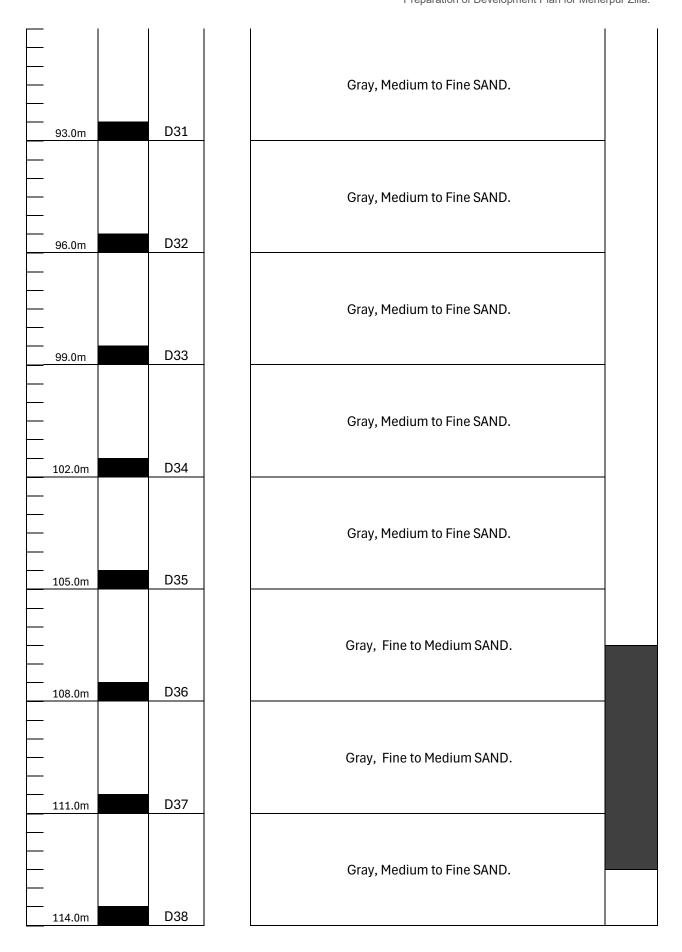


Project: Hydrogeolo	gical Surv	eys and S	Studies under Development Plan for Meherpur Zilla						
	Client: Urban Development Directorate (UDD)								
Bore Hole ID: MW-0	7D								
Location: Motmura	Govt. Prim	narv Scho	ool, Gangni, I	-					
Co-ordinate: 23.898			CENTER FOR						
Depth of Boring: 114 Meter									
Ground Water Leve									
Method of Boring: R		h Boring							
Boring Diameter: 1.									
Date: 19/03/2025									
Depth Bellow GL (m)	Sample No	Thickness (m)		Layer Change					
		F	Lithologic Description						
	D1		Brownish Gray, Silty CLAY						
6.0m	D2		Brownish Gray, Silty CLAY						
9.0m	D3		Gray, Very Fine to Fine SAND with SILT						
12.0m	D4		Gray, Very Fine to Fine SAND with SILT						
15.0m	D5		Gray, Very Fine to Fine SAND with SILT						
18.0m	D6		Gray, Very Fine to Fine SAND with SILT						

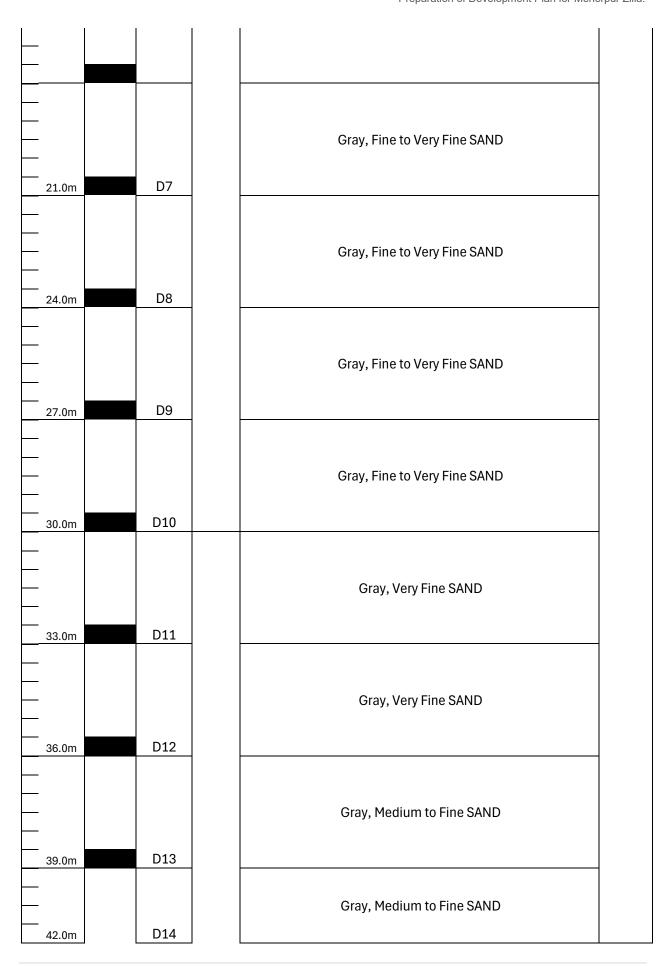


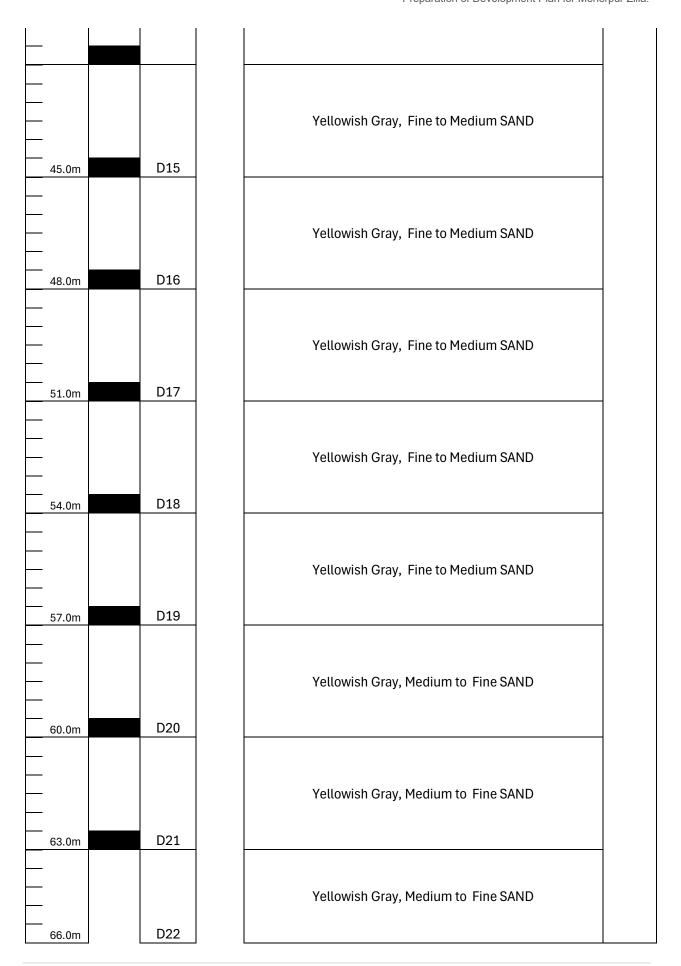


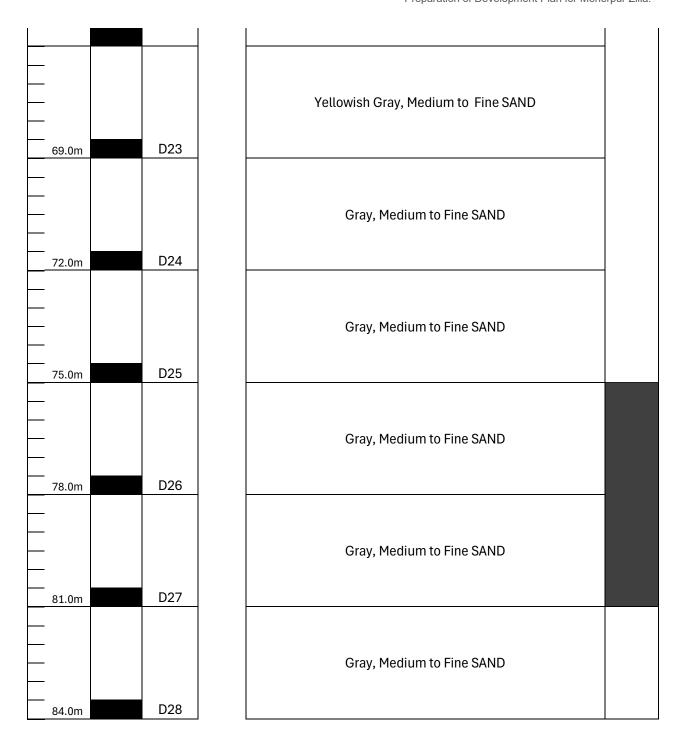




Project: H	lvdrogenl	ngical Sur	vevs and	d Studies under Development Plan for Meherpur Zilla					
Client: Ur									
Bore Hole			5 5 6 1 6	()					
				hool, Gangni, I					
Co-ordinate: 23.898126, 88.834600									
Depth of Boring: 84 Meter									
Ground W									
Method o			sh Borin	g					
Boring Dia		.5"							
Date: 21/		0			0				
Depth Bellow GL (m)	Type of Sample	Sample No	Thickness (m)		Layer Change				
De	Тур		Т	Lithologic Description	Ľ				
				Brownish Gray, Silty CLAY					
_				2.0					
_		D.1							
3.0m		D1							
<u> </u>									
_									
_				Brownish Gray, Silty CLAY					
_									
6.0m		D2							
				Gray, Fine to Very Fine SAND					
				Oray, Time to very Time SAND					
<u> </u>									
9.0m		D3							
<u> </u>									
_									
\vdash				Gray, Very Fine to Fine SAND with SILT					
<u> </u>									
12.0m		D4							
				Cray Vany Eina to Eina CAND with CILT					
				Gray, Very Fine to Fine SAND with SILT					
15.0m		D5							
_				Onco Versell to El CANO SILOUT					
<u></u>		D.C.		Gray, Very Fine to Fine SAND with SILT					
18.0m		D6							







Project: Hydroge	ological Sui	vevs and	d Studies under Development Plan for Meherpur Zilla						
Client: Urban De									
Bore Hole ID: MV		55.676							
Location: Motmu			hool, Gangni, I						
Co-ordinate: 23.898126, 88.834600									
Depth of Boring. 30 Meter									
Ground Water Le	vel: Meter								
Method of Boring		sh Borin	g						
Boring Diameter:	1.5"								
Date: 23/03/202									
Depth Bellow GL (m)	Type of Sample	Thickness (m)		Layer Change					
De	dyl	Тħ	Lithologic Description	Lá					
3.0m	D1		Brownish Gray, Silty CLAY						
3.0111									
6.0m	D2		Brownish Gray, Silty CLAY						
9.0m	D3		Gray, Fine to Very Fine SAND						
12.0m	D4		Gray, Silty CLAY						
15.0m	D5		Gray, Silty CLAY						
18.0m	D6		Gray, Very Fine to Fine SAND						

